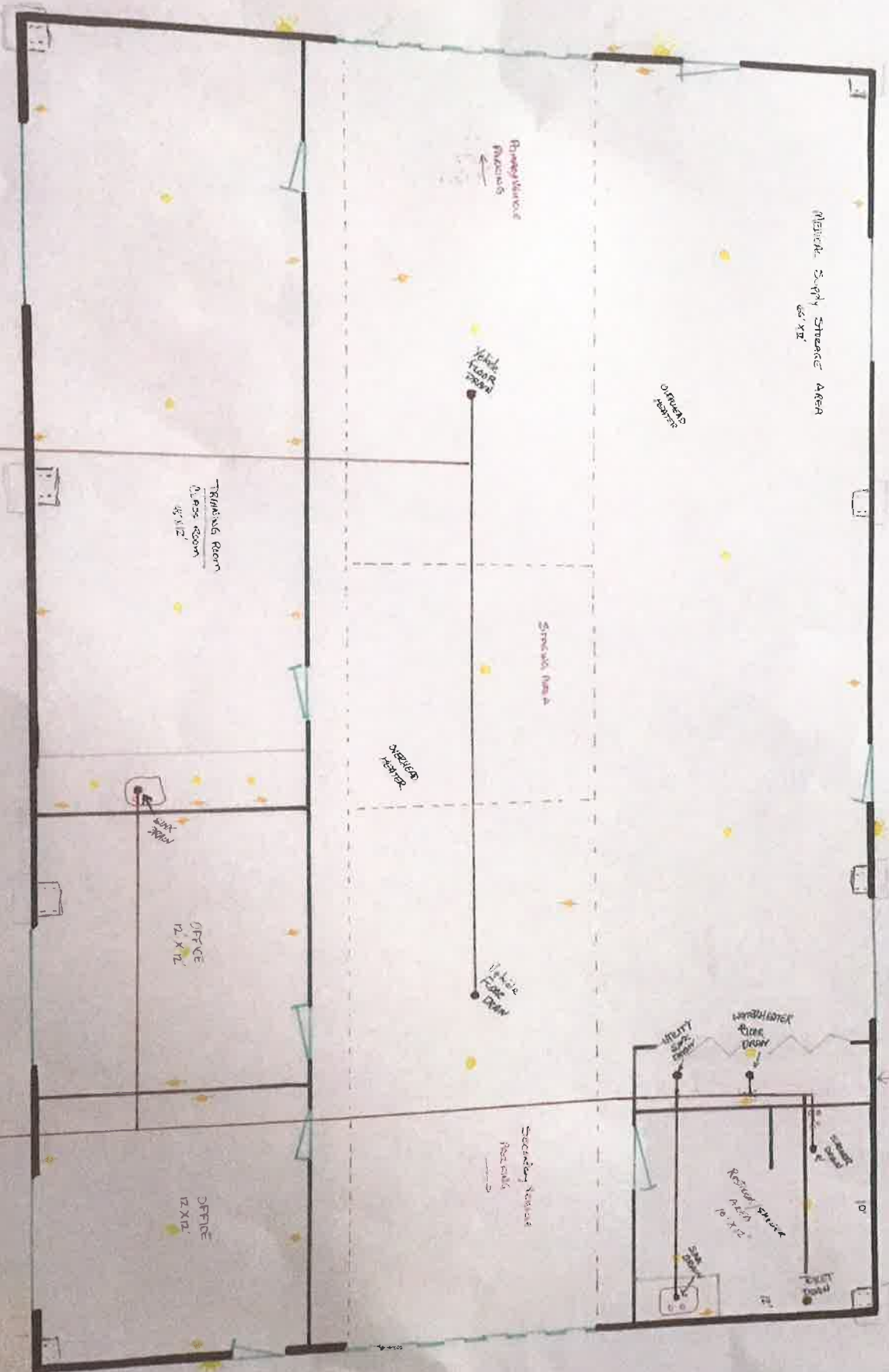


AVAD
 CALDALE STATION
 11998 OR 45 CALDALE, Co. 81222



TO SHOW OR STORAGE TRUCK
 TO SHOW TRUCK

TOILETS
 SINKS
 MIRRORS
 DONORS

Building requirement:

Building is to be insulated according to Fremont County standards which are blanket insulation with an R-10 factor around slab; R-13 factor on all exterior walls; and an R-30 factor on ceiling.

Interior walls shall be 2x4 construction with 5/8 inch dry wall throughout, 16" on center.

Ceiling Joist w/storage-unbraced. Spaced 12', #2 Fir, 16" on center, 2x6.

Overhead propane heaters (two) will heat the building.

Interior doors 36" (5). Bi-fold doors at utility area, exterior doors (3) 36".

Plumbing. To septic system tank all drains from bathroom, utility closet and training room. To floor drain storage tank all vehicle floor drains.

Lighting: all uniform in design, switched to designated areas of use. Offices, rest room and training room on their own switch at entry door. Bay area doors with switches to supply area, primary vehicle area, secondary vehicle area, utility area, outside lighting.

Foundation design provided.

Septic design provided.

Plot plan provided.

East side driveway of concrete, rest of parking and access will be gravel.

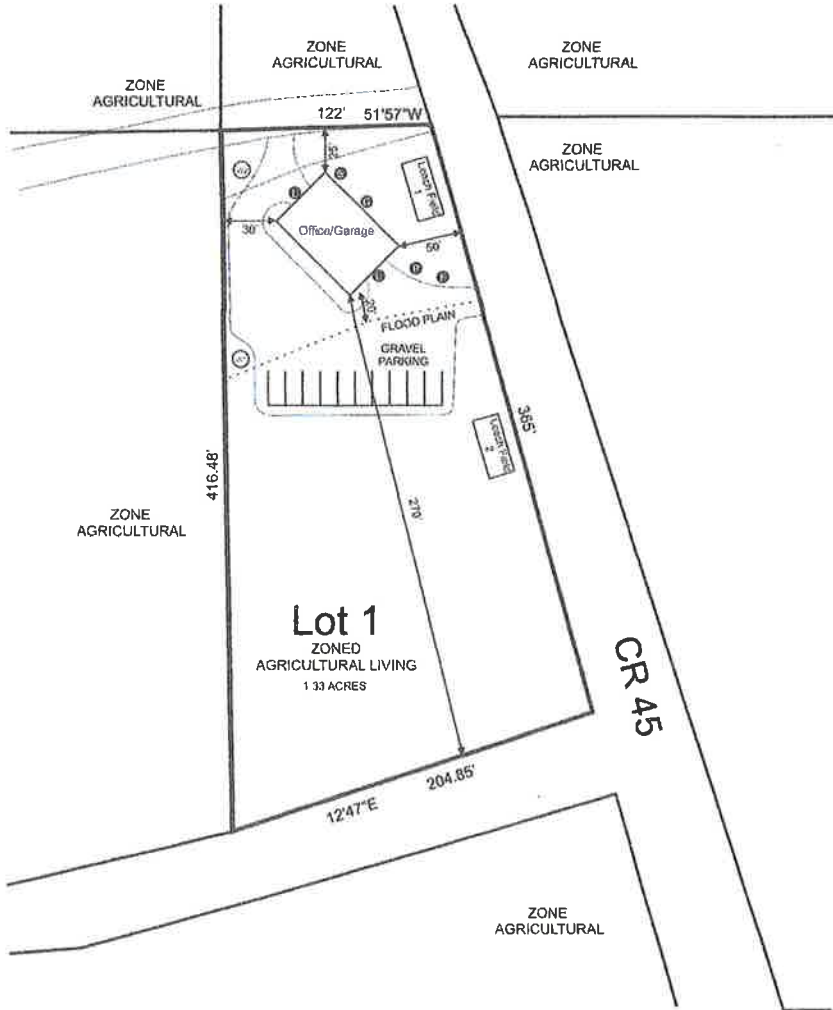
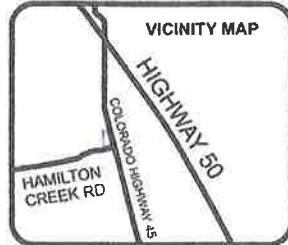
Please advise Board of any concerns or questions at (719) 371-3893.

Arkansas Valley Ambulance District Coaldale Substation

11998 County Road 45, Coaldale, CO 81222
EMERGENCY SERVICE STATION



SCALE 1" = 50'

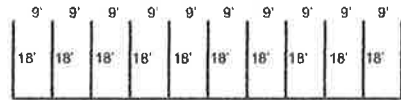


LEGEND

- SITE BOUNDARY ———
- FLOOD PLAIN ·····
- PROPERTY LINES ———
- STRUCTURE []
- GREY WATER TANK (G)
- SEPTIC TANK MAN HOLE (S)
- ELECTRIC AND PHONE (E/P)
- EXTERIOR LIGHTING (L)
- PROPANE TANK (P)
- 500 GAL. FUEL TANK (F)
- WITH SECONDARY CONTAINMENT

Soil Number	Soil %	Description
10	10.8%	bronell gravelly sandy loam, 2 to 15% slopes
74	89.2%	mussefs-bronell complex, 2 to 15% slopes

PARKING DETAIL 1"=20'



Number of Parking Spots **10** Standard 9'x18'

PARKING TABLE

Use	Required Spaces	Proposed Spaces	Explanation
Emergency Service Station (2400sq. ft.)	10	10	Includes ambulance employee spaces

LEGAL DESCRIPTION

A tract of land located in the Northwest One-Quarter (NW ¼) of the Southeast (SE ¼) of Section 29, Township 48 North, Range 11 east of the New Mexico Principle Meridian, County of Fremont, State of Colorado, being more particularly described as follows.

Beginning at the Center one-Quarter (¼) corner of said Section 29; thence S 89° 37' 27" E, a distance of 96.38feet to a point on the Westerly right of way line of Fremont County Road 45; thence S 15° 23' 15" W along said County Road, a distance of 368.71 feet to a point on the Northerly right of way line of Hamilton Creek Road; thence S 72° 51' 57" W along said Hamilton Road, a distance of 416.48 feet to a pint on the North-South centerline of said section; thence N 00° 12'47" E, a distance of 416 feet to the point of beginning.

Area:	Facility Setbacks:	Heights:
Overall 57,819 SF 100%	North 25.0'	Steel Building 20'
Building 2400 SF 4.1%	West 30.0'	
Existing 0 SF	East 50.0'	
Proposed 2400 SF	South 270.0'	

Address: 11998 County Road 45 Howard, Colorado 81240
Existing Zone: Agricultural Living
Existing use: Agricultural living

Uses:
4.3.4.10 Emergency Services Station

Site Notes:

1. No internal roadways are proposed.
2. No pedestrian walkways are planned.
3. No landscaping is proposed.
4. Off-street parking area lighting is provided by a light on the northwest & Southeast sides of the structure. No additional lighting is proposed.
5. No known easements are located on the subject property.
6. No proposed easements are anticipated.
7. The site is located in a FEMA flood area.
8. Significant natural features are not on the property.
9. Drainage structures are not located on the property and are not anticipated.
10. No Refuse, Daily carry out.
11. No phone line. Relying on cell service

Proposed Structures table

SHOP/OFFICE 40'X60' 2,400 SF BUILDING HEIGHT 20'

DRAWN BY: J.M. SCHLUP	APPROVED BY:
DATE: 2/19/2022	PROV. NO.
SURVEY NO. 457	DWG.

REVISIONS: 2/1/2022

PROJECT: ARKANSAS VALLEY AMBULANCE DISTRICT 11998 COUNTY ROAD 45 FREMONT COUNTY	SHT. NAME: COMMERCIAL DEVELOPMENT PLAN	SHT. NO: 1 OF 1
--	--	-----------------

NOTES AND SPECIFICATIONS:

FIELD SIZE AND CALCULATIONS:

- BASED ON NUMBER OF EMPLOYEES: 150 GAL/DAY
- NUMBER OF BATHROOMS: 1 (1 SHOWER, 1 TOILET, 2 SINKS)
- NUMBER OF KITCHENS: 1 KITCHENETTE
- LONG TERM ACCEPTANCE RATE: 0.3 GAL / SQ. FT. / DAY
- SITE AND SOIL EVALUATION BY: JESK CONSULTING
- ENGINEERED FIELD TYPE: TRENCH WITH INFILTRATORS
- REQUIRED # OF QUICK 4's: 18 QUICK 4's

GENERAL SYSTEM NOTES:

- REFER TO LOCAL CODES AND REQUIREMENTS BEFORE INSTALLATION
- SYSTEM MUST BE INSTALLED BY QUALIFIED AND LICENSED INSTALLER
- ANY PORTION OF THE ABSORPTION FIELD MUST BE AT LEAST 150" FROM ANY WATER WELL
- SYSTEM MUST BE INSTALLED WITH THE INDICATED NUMBER OF INFILTRATORS, ZONES, AND PIPE LENGTH UNLESS SPECIFIC WRITTEN APPROVAL IS OBTAINED BY THE DESIGN ENGINEER
- ALL LATERALS MUST BE INSTALLED LEVEL, INDIVIDUAL ZONES MAY BE INSTALLED AT DIFFERENT ELEVATIONS. SEE FIELD CROSS SECTION FOR ADDITIONAL INFORMATION.
- A SEWER CLEAN-OUT MUST BE INSTALLED OUTSIDE THE STRUCTURE AND WITHIN FIVE FEET OF THE SEWER EXIT FROM THE FOUNDATION
- NOT ALL COMPONENTS ARE SPECIFICALLY SHOWN ON THESE PLANS (ELBOWS, VENTS, VALVES, ETC.) AND IT IS ASSUMED THE INSTALLER IS FAMILIAR WITH THE STANDARDS FOR SYSTEM INSTALLATIONS FOR THESE NON-SPECIFIED COMPONENTS.
- THE ABSORPTION FIELD MAY NOT BE USED FOR ANY ACTIVITIES THAT MAY COMPACT THE SOIL, FLOOD THE FIELD, DAMAGE THE PIPES, OR NEGATIVELY IMPACT THE OPERATION OF THE FIELD IN ANY MANNER (LIVESTOCK AREAS, VEHICLE TRAFFIC, CONSTRUCTION AREAS, STORAGE AREAS, ETC.)

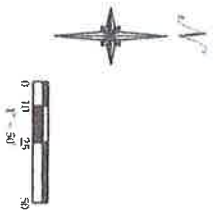
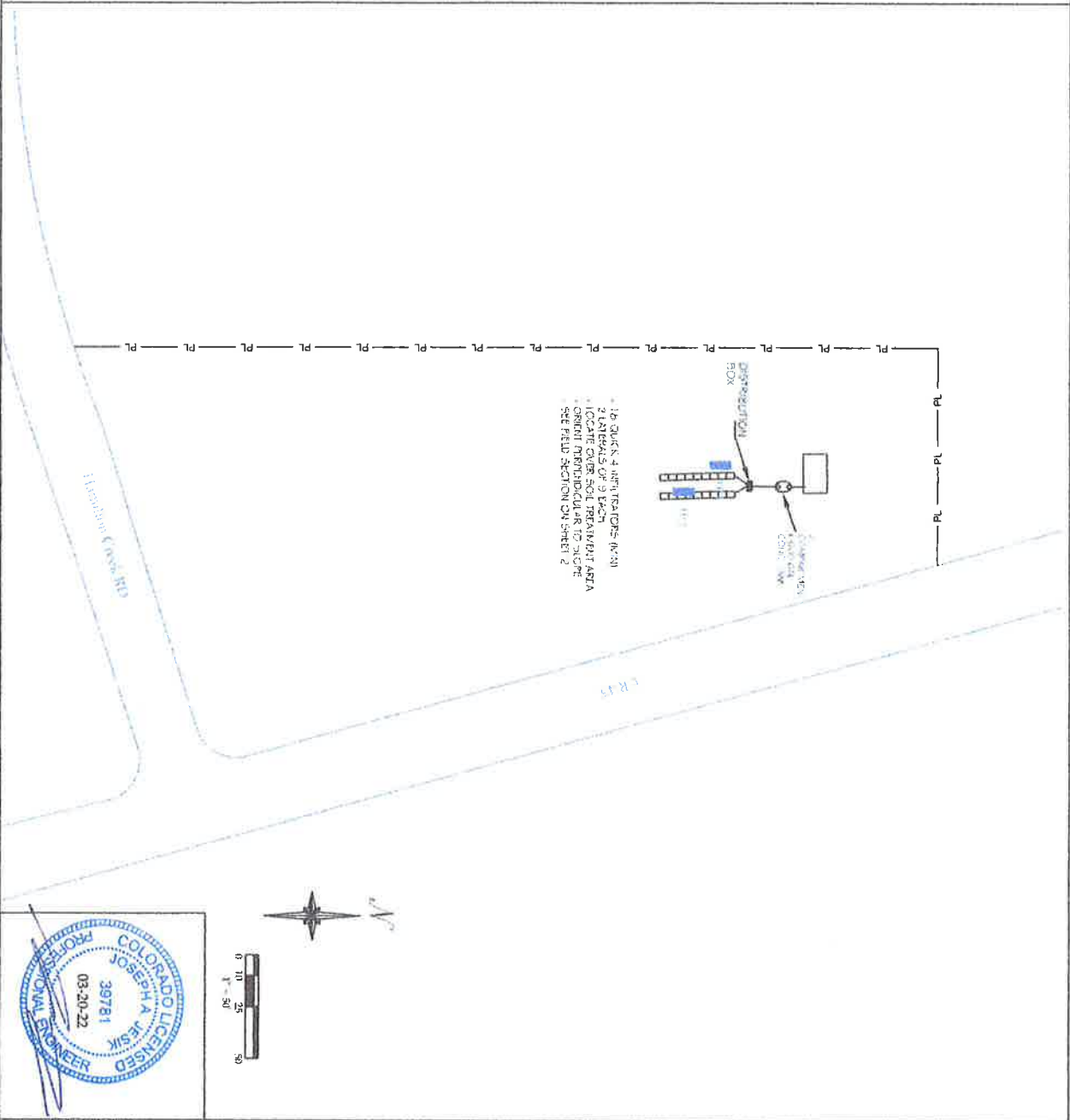
TANK NOTES AND SPECIFICATIONS:

CERTIFICATION:

TANKS SHOULD BE APPROVED BY THE COLORADO DEPARTMENT OF HEALTH AND ENVIRONMENT (CDPHE).

GENERAL TANK NOTES & SPECIFICATIONS:

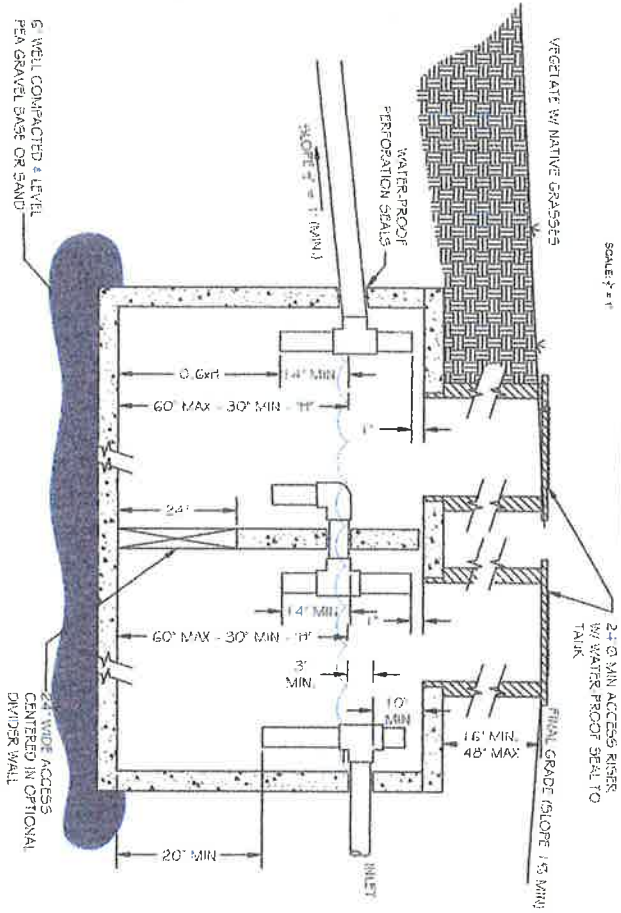
- TANKS MUST BE NO DEEPER THAN 4 FT FROM THE TOP OF TANK TO THE GROUND SURFACE
- TOP OF TANKS MUST BE DESIGNED AND CONSTRUCTED TO SUPPORT A MIN 400 PSF UNIFORM LOAD PLUS 2,500 POUND AXLE LOAD, WHEN BURIED MORE THAN 2 FT DEEP. THE TANK SHALL SUPPORT AN ADDITIONAL, 100 PSF PER EA FT OF SOIL, DEEPER THAN 2 FT
- TANKS MUST BE INSTALLED PER LOCAL CODE AND THE MANUFACTURERS INSTRUCTIONS.



	<p>ARK VALLEY AMBULANCE</p> <p>OWTS PLAN FOR: 11998 COUNTY ROAD 45 FREMONT COUNTY, COLORADO</p>	<p>800 W. 8TH STREET PUEBLO, COLORADO 81003 (719) 562-5588 www.jesik.us</p>
<p>OW1</p>	<p>Drawn By: JJP Date: 2/25/2022 Proj No: 21-8970 Rev No: REV 0 Scale: 1" = 50'</p>	

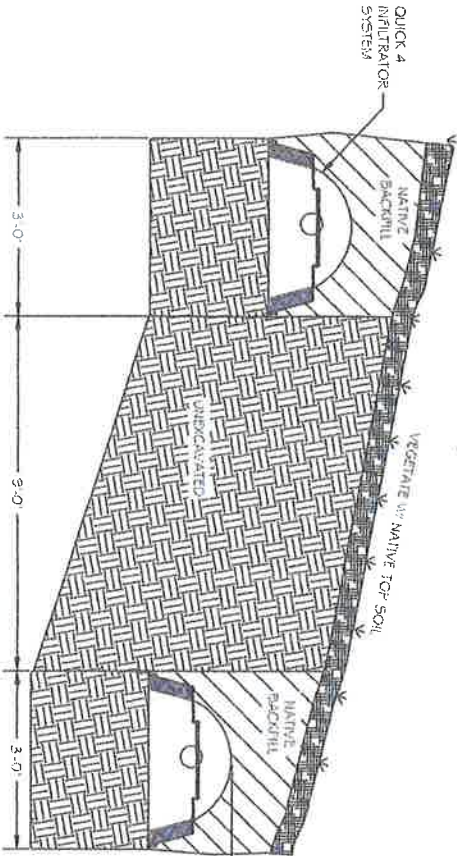
1,000 GALLON TANK

SCALE: 1" = 1'



FIELD CROSS SECTION

SCALE: 1" = 1'

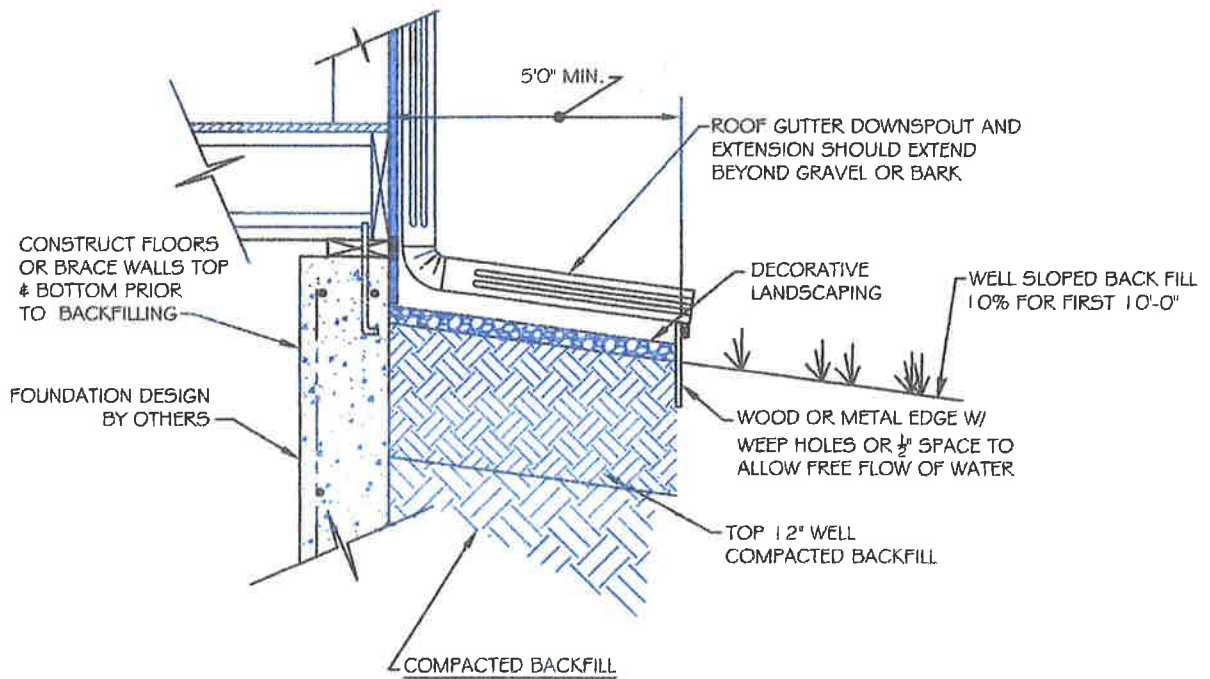


12" MIN. COVER TYP. - MOUND SOIL IF NEEDED
 MAX. DEPTH = 12" BELOW EX. GROUND
 DO NOT PLACE INFILTRATOR BOTTOM HIGHER THAN EX. GROUND



OW2	Project No. 20250222 Date: 03-20-22 Job No. 21-8970 Rev. No. REV. 0 Sheet No. T = 50	ARK VALLEY AMBULANCE OWT'S PLAN FOR: 11998 COUNTY ROAD 45 FREMONT COUNTY, COLORADO	 880 W. 8TH STREET PUEBLO, COLORADO 81003 (719) 582-5388 www.jesik.us
	11998 COUNTY ROAD 45 FREMONT COUNTY, COLORADO		

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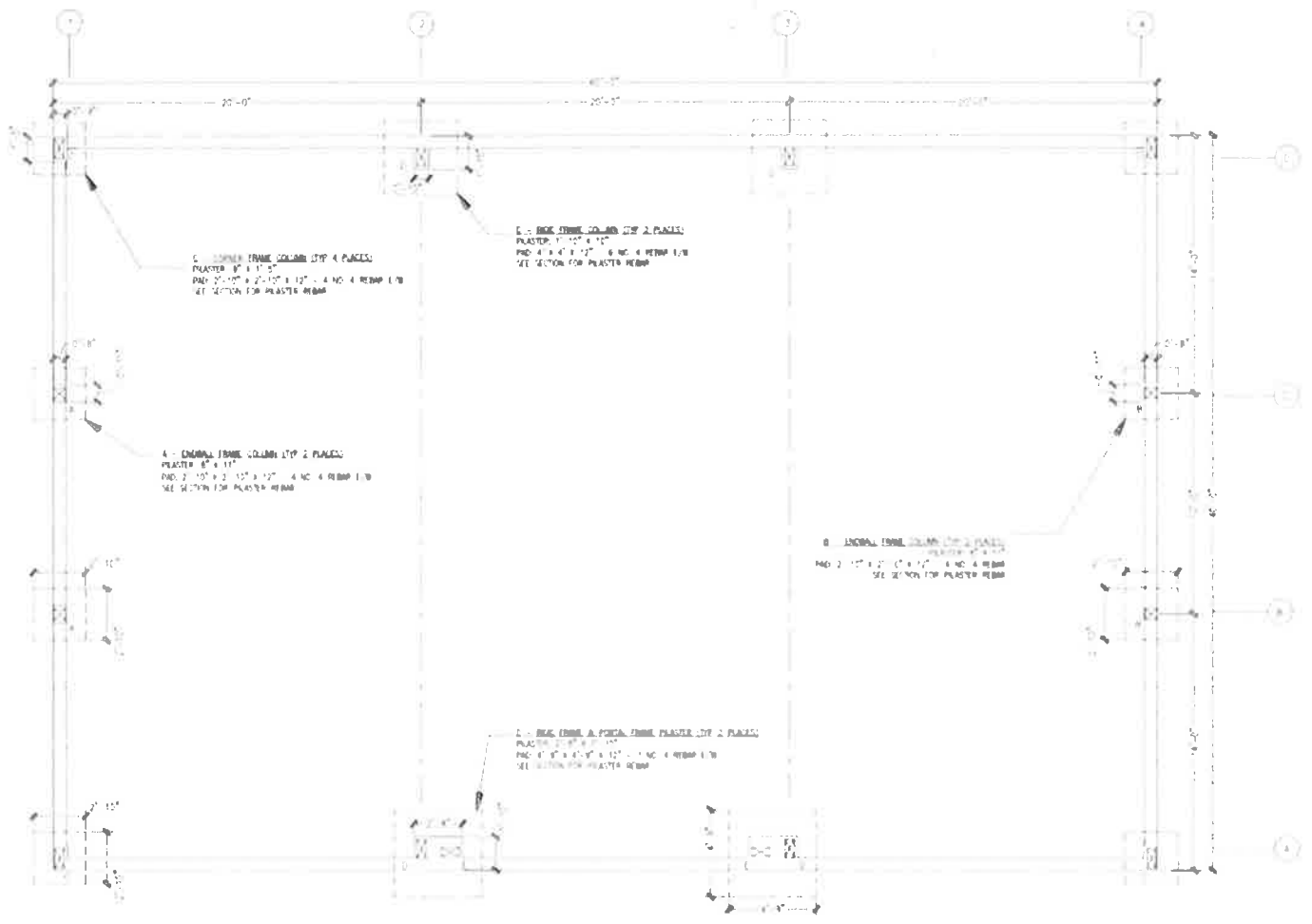
-DO NOT OVER WET OR PUDDLE WATER DURING BACKFILL (SEE REPORT FOR CAUTION ON HYDROSTATIC LOADS)

-DO NOT ALLOW LARGE MACHINE TRAFFIC DURING BACKFILL

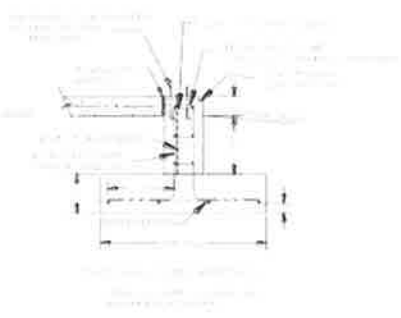
BACKFILL AROUND THE FOUNDATION SHOULD BE MOISTURE CONDITIONED AND WELL COMPACTED. THE FINAL GRADE SHOULD BE SLOPED TO PREVENT PONDING OF WATER ADJACENT TO FOUNDATION WALLS.

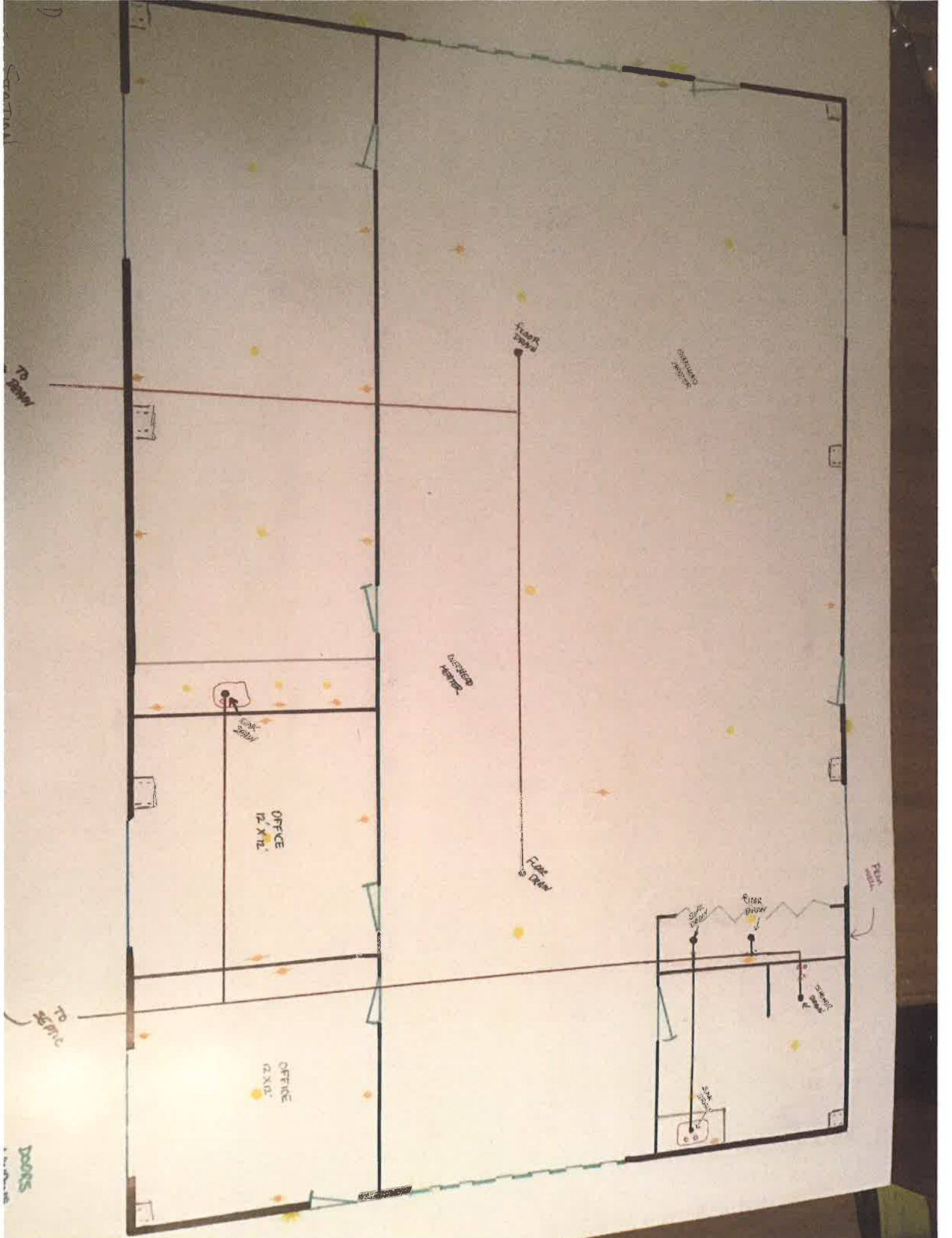
BACKFILL SHOULD NOT CONTAIN ROCKS OVER 6" IN DIAMETER OR ANY CONSTRUCTION DEBRIS.

DO NOT DIKE OR IMPEDE THE FLOW OF WATER AWAY FROM FOUNDATION WALLS. DOWNSPOUTS AND SILL COCKS SHOULD DISCHARGE A MIN. OF 5' FROM THE FOUNDATION.



FOUNDATION PLAN
 REFER TO MANUFACTURER PLAN FOR FOUNDATION BRICKS, REB. SIZES AND OTHER DETAILS
 VERIFY ALL DIMENSIONS
 CENTER PLASTERS A, B, C, E, AND D1 ON FOOTING PAD





GENERAL NOTES

- 1.1 Fabrication shall be in accordance with A.S.C. standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3".
- 1.2 **MATERIALS**
- | ASTM DESIGNATION | MIN. YIELD STRENGTH |
|---------------------------------------|--------------------------------------|
| Hot Rolled Steel Shapes (W. & C) | A572
Fy = 50 KSI |
| Hot Rolled Steel Angles (L) | A36
Fy = 36 KSI |
| Steel Pipes | A500
Fy = 42 KSI |
| Structural Tubing | A500
Fy = 42 KSI |
| Structural Steel Web Plate | A572/A1011
Fy = 50 KSI |
| Structural Steel Flange Plates/Bars | A529/A572
Fy = 55 KSI |
| Cold Formed Light Gage | A653/A1011
Fy = 55 KSI |
| Roof and Wall Sheets | A792/A653
Fy = 50, 80 KSI |
| Cable Braces | A475 - TYPE 1
Extra High Strength |
| Rod Braces | A36
Fy = 36 KSI |
| MIN. TENSILE STRENGTH | |
| Machine Bolts & Nuts | A307
Fu = 60 KSI |
| High Strength Bolts (1" and less) | A325-TYPE 1
Fu = 120 KSI |
| High Strength Bolts (>1" to 1 1/2") | A325-TYPE 1
Fu = 105 KSI |
| Anchor Bolts (Not supplied by A.S.C.) | A36/A307/F1554
Fu = 60 KSI |
- 1.3 **PRIMER**
Shop primer point is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is A.S.C. Gray Oxide color. This point is not intended for long term exposure to the elements. A.S.C. is not responsible for any deterioration of the shop primer point as a result of improper handling and/or jobsite storage. A.S.C. shall not be responsible for any field applied point and/or coatings. (AISC Code of Standard Practice, Latest Edition). Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.
- 1.4 **GALVANIZED OR SPECIAL COATINGS:**
See Contract Documents
- 1.5 **ALL BOLTS ARE 1/2" x 0'-1 1/4" A307 EXCEPT:**
a) Endwall rafter splice - 5/8" x 0'-1 3/4" A325-N
b) Endwall column to rafter connection - 1/2" x 0'-1 1/4" A325 MIN. (SEE WALL ELEVATION)
c) Main frame connections - SEE CROSS SECTION
d) Flange Brace connections - 1/2" x 0'-1 1/4" A325
NOTE: Washers are not supplied unless noted otherwise on drawing
- 1.6 **A325 BOLT TIGHTENING REQUIREMENTS**
All high strength bolts are A325-N unless specifically noted otherwise. Holes are not slotted and design is bearing connection. Structural bolts shall be tightened by the turn-of-the-nut method in accordance with the Latest Edition AISC "Specification For Structural Joints" using ASTM A325 or A490 Bolts, when specifically required. A325-N bolts are supplied without washer unless otherwise noted on the drawings. All bolted connections unless noted are designed as bearing type connections with bolt threads not excluded from the shear plane.
- 1.7 **CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:**
INSIDE - Under roof panels & base of wall panels
OUTSIDE - Between roof panels & ridge cap
- Between wall panels & eave/gable trim
- 1.8 **ERECTION NOTE:**
All bracing, strapping, & bridging shown and provided by A.S.C. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.
- 1.9 **ERECTION AND UNLOADING NOT BY A.S.C.**
- 1.10 **SHORTAGES**
Any claims or shortages by buyer must be made to A.S.C. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.
- 1.11 **CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)**
Claims for correction of alleged misfits will be disallowed unless A.S.C. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of A.S.C.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- 2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release A.S.C. to fabricate upon receiving such.
- 2.2 Armstrong Steel Corp (hereafter referred to as A.S.C.) standard specifications apply unless stipulated otherwise in the Contract Documents. A.S.C. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.
- 2.3 In case of discrepancies between A.S.C. structural steel plans and plans for other trades, A.S.C. plans shall govern. (Section 3 AISC Code of Standard Practices, Latest Edition)
- 2.4 Approval of A.S.C. drawings and calculations indicates that A.S.C. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the A.S.C. design concepts, assumptions, and loading. (Section 4 AISC Code and MBMA 3.3.3)
- 2.5 Once the BUYER/END USE CUSTOMER has signed A.S.C. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.



PHONE: 800-345-4610
www.armstrongsteel.com

JOB NO. : 55101

CUSTOMER : CHRIS BERTOLINO AND/OR POST SCRIPTS LLC

END USER : CHRIS BERTOLINO

END USE : WORKSHOP

LOCATION : 18 ALTA VISTA RD

: COTOPAXI, CO 81223

: FREMONT COUNTY

PH. NO. : 409-392-5658 EMAIL: POSTSCRIPTSLLC@GMAIL.COM

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

Design Code / Wind Code	: IBC-15
Building Risk Category	: II - Normal
Enclosure	: Closed
Dead Load (psf)	: 2.00
Collateral Load (psf)	: 2.00
Wind Load	
Ultimate Wind Speed, (Vult) (mph)	: 145.00
Wind Exposure	: C
Internal Pressure Coefficient, GCpi	: 0.18/-0.18
Wall Panel Design Wind Pressure (psf)	: 46.7/-50.7
Live Load	
Primary Framing (psf)	: 20.00
Trib. Area Reduction	: No
Secondary Framing (psf)	: 20.00
Snow Load	
Ground Snow Load, Pg (psf)	: 45.00
Roof Snow Load, Pf (psf)	: 45.00
Sloped Roof Snow Load, Ps (psf)	: 45.00
Snow Exposure Factor, Ce	: 1.00
Snow Importance Factor, Is	: 1.00
Thermal Factor, Ct	: 1.20
Sloped Factor, Cs	: 1.00

Seismic Load

Seismic Importance Factor, Ie	: 1.00
Site Class	: D
Mapped Spectral Response Acceleration	: Ss = 0.256 :S1 = 0.081
Spectral Response Coefficients	: Sds = 0.272 :Sd1 = 0.130
Seismic Design Category	: B
Basic Force Resisting Systems Used	: Steel System Not Specifically Detailed For Resistance
	: Rigid Frames (OMF)
	: Braced Frames (OCBF/OMF)
Total Design Base Shear, V (kips)	: Longitudinal = 3.62
	: Transverse = 3.32

Response Modification Factors, R	: Rigid Frames = 3.50	$\Omega = 3.00$
	: SW X-Bracing = 3.25	$\Omega = 2.00$
	: SW Wind Bent = 3.50	$\Omega = 3.00$

Seismic Response Coefficient, Cs	: Rigid Frames = 0.0778
	: SW X-Bracing = 0.0838
	: SW Wind Bent = 0.0778

Analysis Procedure Used	: Equivalent Lateral Force Procedure
Other Loads/Requirements	

BUILDING DESCRIPTION:

Width (ft)	: 40	Insulation	
Length (ft)	: 60	Roof Insulation	: None
Eave Ht. at BSW (ft)	: 14	Wall Insulation	: None
Eave Ht. at FSW (ft)	: 14		
Roof Slope at BSW	: 3:0:12		
Roof Slope at FSW	: 3:0:12		
Bay Spacing (ft)	: 3 at 20		

COVERING AND TRIMS:

Roof Panels & Trims	
Panel Type	: 26 Ga. R-Loc
Panel Color	: Galvalume Plus
Trim Colors	
Gable/Eave Trim	: Lt. Stone 40 yr
Wall Panel & Trims	
Panel Type	: 26 Ga. R-Loc
Panel Color	: Desert 40 yr
Trim Colors	
Corner Trims	: Lt. Stone 40 yr
Opening Trims	: Lt. Stone 40 yr
Base Trim	: Desert 40 yr

Drawing Index

Drawing Name	Page(s)
Drawing Cover	---
Anchor Bolt Plan	1
Anchor Bolt Details	2
Anchor Bolt Reactions	3
Rigid Frame	4-7
Front Sidewall	8
Back Sidewall	9
Left Endwall	10
Right Endwall	11
Roof Plan	12
Details	13-16

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT ARMSTRONG STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY A.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN ARMSTRONG ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

- 2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any material not furnished by A.S.C. and A.S.C. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or A.S.C. assumptions will govern (AISC Code of Standard Practice, Latest Edition)
- 2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that A.S.C. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that A.S.C. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by A.S.C.
- 2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with A.S.C. "For Construction" drawings only. Temporary supports such as gys, braces, falsework, cribbing or other elements required for the erection operation shall be determined furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (AISC Code of Standard Practice, Latest Edition.)
- 2.9 Armstrong Steel Corp is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete. Unless otherwise provided in the Order Documents, A.S.C. does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site. It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Latest MBMA Low Rise Building Systems Manual)
- 2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to A.S.C. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (AISC Code of Standard Practice Latest Edition)
- 2.11 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, sizes, location and number of alterations prior to preparation of shop drawings. (AISC Code of Standard Practice Latest Edition)
- 2.12 **WARNING:** In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto Galvalume should be avoided.
- 2.13 **SAFETY COMMITMENT:** Armstrong Steel Corp has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of A.S.C. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.
- 2.14 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.
- 2.15 It is recommended by Factory Mutual (Reference: 82.44) that roofs be cleared of snow when half of the maximum snow depth is reached. The maximum snow depth can be estimated based on the design snow load and the density of snow and/or ice buildup. See Chart below.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

NOTE:
For Snow/Ice Removal Procedures, Refer to Metal Building System Manual 2002 Edition, Section AB.4, Page XI-AB-2.

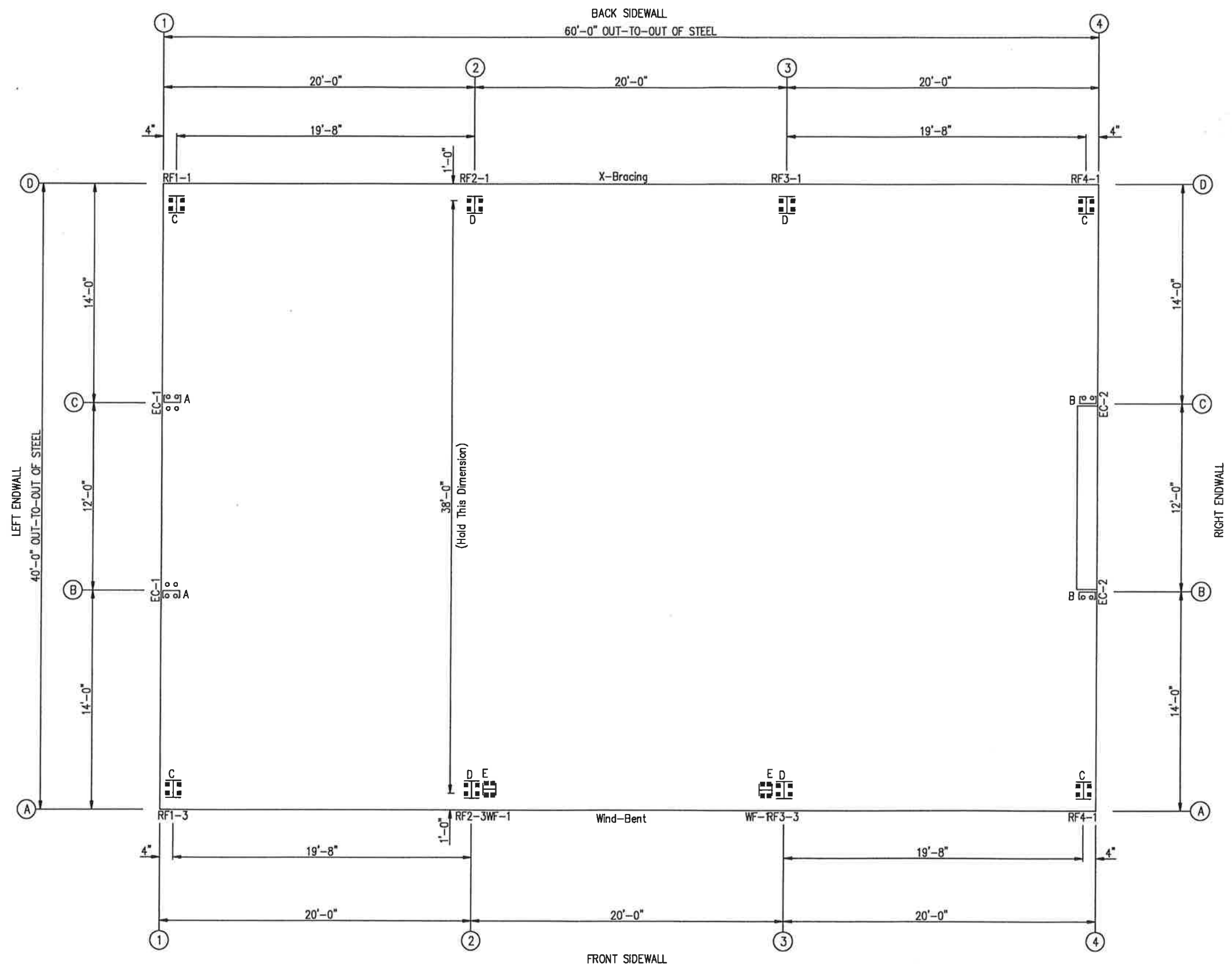
Drawing Status

- APPROVAL:** **REVISED APPROVAL:**
These drawings are conceptual only and are not to be used for the permit or construction process.
- PERMIT/CONSTRUCTION:** **REVISED PERMIT/CONSTRUCTION:**
These drawings are Final and are for review by the building official or others.
- ERECTION DRAWINGS:**
Drawings to be used for the erection of the building.

JOB NO : 55101 CHRIS BERTOLINO AND/OR POST SCRIPTS LLC

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
○ 12	Endwall	5/8"	A307
⊗ 32	Frame	3/4"	A307
⊗ 8	WindCol	3/4"	A307



NOTE:
 THIS DESIGN IS BASED ON RIGID FRAMES @
 LINE 1 & LINE 2 ARE TO SUPPORT FUTURE LEAN-TO
 10' WIDE X 20' LONG X 10' LOW EAVE HT/12'-6"
 HIGH EAVE HT, 3:12

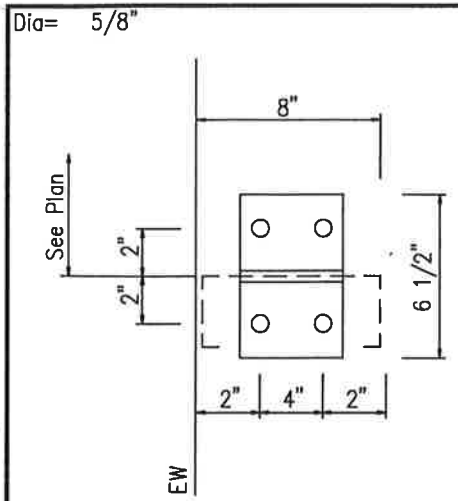
NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY
 AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
 ENSURE PROPER FIT. SUCH WORK IS CONSIDERED
 A NORMAL PART OF METAL BUILDING ERECTION.
 A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR
 FIELD WORK.

ANCHOR BOLT PLAN
 NOTE: All Base Plates @ 100'-0" (U.N.)

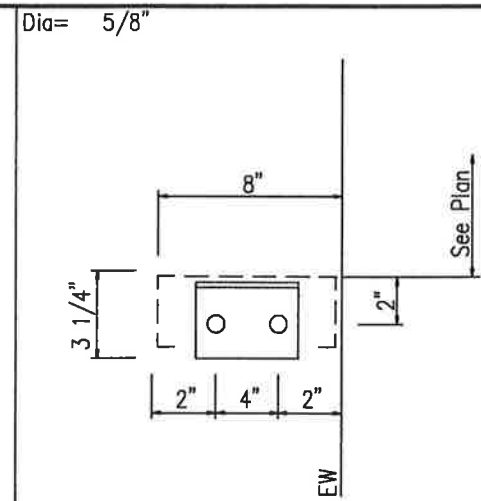
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

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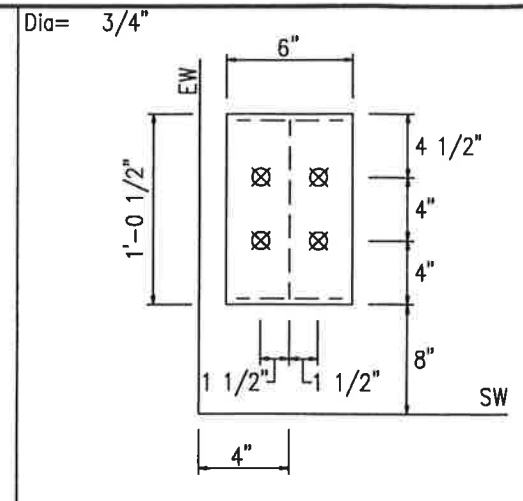
DESCRIPTION	ANCHOR BOLT PLAN
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE
JOB NO.:	55101
ENG. BY:	SS
DATE:	04/25/17
DWG. NO.:	1 OF 16
ISSUE:	E



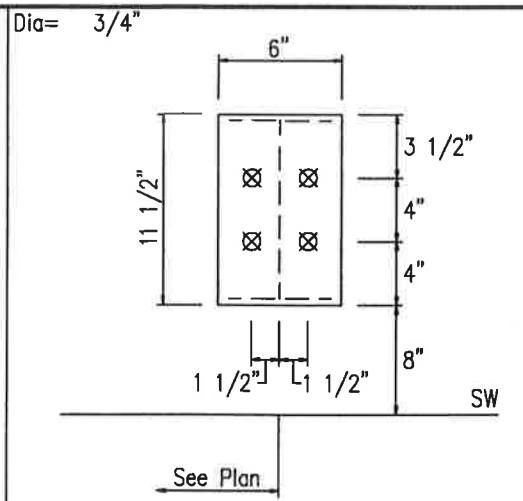
DETAIL A



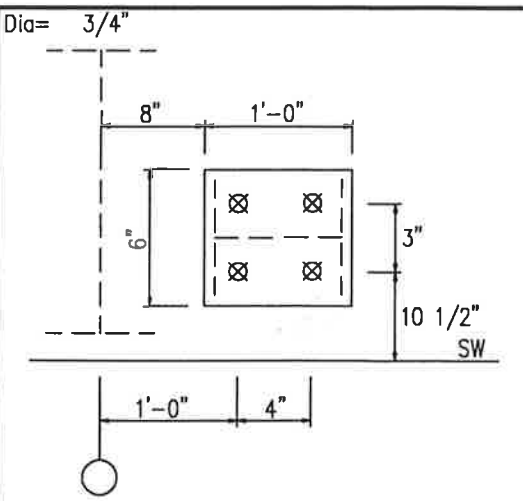
DETAIL B



DETAIL C



DETAIL D



DETAIL E

NOTE:
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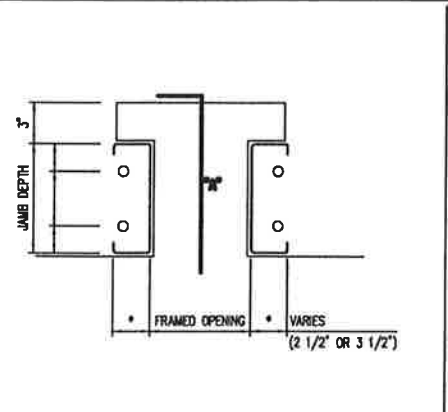
ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING MANUFACTURER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

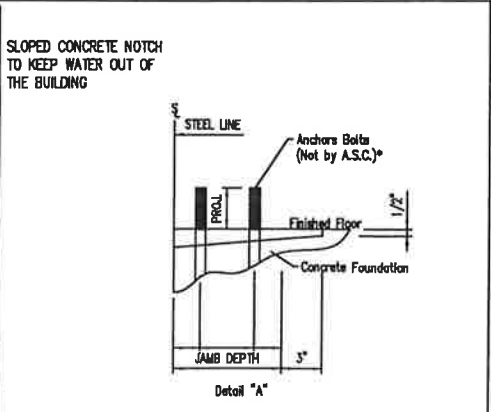
NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.

Anchor Bolt Diameter	Projection
1/2"	1 1/2"
5/8"	2"
3/4"	3 1/2"
7/8"	3 1/2"
1"	3 1/2"
1 1/4"	3 1/2"
1 1/2"	3 1/2"

ANCHOR BOLT PROJECTION



DETAIL AT FRAMED OPENING



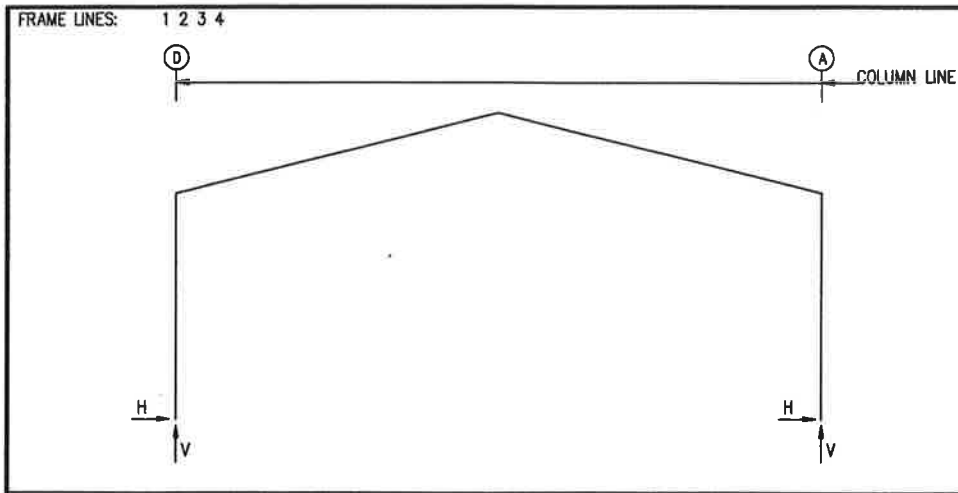
DETAIL "A"

NOTE:
THIS DESIGN IS BASED ON RIGID FRAMES @
LINE 1 & LINE 2 ARE TO SUPPORT FUTURE LEAN-TO
10' WIDE X 20' LONG X 10' LOW EAVE HT/12'-6"
HIGH EAVE HT, 3:12

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

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DESCRIPTION	ANCHOR BOLT DETAILS
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE
JOB NO.:	55101
ENG. BY:	SS
DATE:	04/25/17
DWG. NO.:	2 OF 16
ISSUE:	E



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
1	D	1	3.6	10.4	3	-3.4	-5.3	4	0.750	6.000	12.50	0.500	0.0
1	A	6 1	3.2 -3.5	-4.3 12.8	2 4	-3.6 2.8	7.3 -6.0	4	0.750	6.000	12.50	0.500	0.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2	D	1	8.7	20.2	3	-5.2	-8.0	4	0.750	6.000	11.50	0.500	0.0
2	A	6 1	4.9 -8.6	-5.3 22.6	7 1	-0.5 -8.6	-8.6 22.6	4	0.750	6.000	11.50	0.500	0.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
3	D	1	8.7	20.2	3	-4.7	-7.6	4	0.750	6.000	11.50	0.500	0.0
3	A	4 1	4.7 -8.7	-7.6 20.2	1 4	-8.7 4.7	20.2 -7.6	4	0.750	6.000	11.50	0.500	0.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
4	D	1	3.6	10.5	5	-2.9	-3.2	4	0.750	6.000	12.50	0.500	0.0
4	A	6 1	2.9 -3.6	-3.2 10.5	1 4	-3.6 2.8	10.5 -4.9	4	0.750	6.000	12.50	0.500	0.0

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Wind Press Horz	Wind Suct Horz
1	C	0.1	-3.8	4.2
1	B	0.1	-3.8	4.2
4	B	0.1	-3.8	4.2
4	C	0.1	-3.8	4.2

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
1	C	8 10	2.5 2.5	0.1 0.1	9	-2.3	0.1	4	0.625	6.500	6.000	0.313	0.0
1	B	8 10	2.5 2.5	0.1 0.1	9	-2.3	0.1	4	0.625	6.500	6.000	0.313	0.0
4	B	8 10	2.5 2.5	0.1 0.1	9	-2.3	0.1	2	0.625	3.250	6.000	0.313	0.0
4	C	8 10	2.5 2.5	0.1 0.1	9	-2.3	0.1	2	0.625	3.250	6.000	0.313	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	D	0.2	0.9	0.1	0.4	1.4	4.0	3.2	9.1	-5.9	-9.8	0.6	-5.7
1	A	-0.2	1.0	-0.1	0.5	-1.4	5.0	-3.1	11.2	-1.9	-5.8	4.8	-11.1

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	D	-5.5	-6.5	1.0	-2.5	-0.9	-7.0	-1.6	-6.2	-0.3	-0.2	0.3	0.2
1	A	-1.2	-2.9	5.6	-8.2	-1.2	-5.5	-1.9	-6.3	-0.4	0.2	0.4	-0.2

Frame Line	Column Line	Seismic_Long		MIN_SNOW		F1UNB_SL_L		F1UNB_SL_R	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	D	0.0	0.0	1.4	4.1	2.5	8.5	2.5	4.9
1	A	0.0	0.0	-1.4	4.1	-2.5	4.9	-2.5	8.5

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	D	0.5	1.4	0.4	0.8	3.5	8.0	7.8	18.0	-9.2	-14.8	0.1	-9.2
2	A	-0.5	1.6	-0.4	0.9	-3.4	8.9	-7.8	20.1	-1.5	-9.3	8.2	-16.1

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	D	-8.5	-8.7	0.8	-3.2	-1.4	-15.8	-2.7	-14.2	-0.5	-0.4	0.5	0.4
2	A	-1.0	-3.6	8.6	-10.4	-0.1	-10.7	-1.4	-12.2	-0.6	0.4	0.6	-0.4

Frame Line	Column Line	Seismic_Long		MIN_SNOW		F2UNB_SL_L		F2UNB_SL_R	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	D	0.0	-0.9	3.5	8.0	6.1	16.7	6.1	9.6
2	A	0.0	0.0	-3.5	8.0	-6.1	9.6	-6.1	16.7

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
3	D	0.5	1.4	0.4	0.8	3.5	8.0	7.9	18.0	-8.3	-14.1	0.1	-9.2
3	A	-0.5	1.4	-0.4	0.8	-3.5	8.0	-7.9	18.0	-0.1	-9.2	8.3	-14.1

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
3	D	-8.1	-8.4	0.3	-3.6	-0.3	-14.9	-1.6	-13.3	-0.5	-0.3	0.5	0.3
3	A	-0.3	-3.6	8.1	-8.4	1.6	-10.4	0.3	-12.0	-0.5	0.3	0.5	-0.3

Frame Line	Column Line	Seismic_Long		MIN_SNOW		F3UNB_SL_L		F3UNB_SL_R	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
3	D	0.0	-0.9	3.5	8.0	6.1	16.7	6.1	9.6
3	A	0.0	0.0	-3.5	8.0	-6.1	9.6	-6.1	16.7

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
4	D	0.2	0.9	0.1	0.4	1.4	4.1	3.2	9.2	-5.0	-9.0	0.6	-5.7
4	A	-0.2	0.9	-0.1	0.4	-1.4	4.1	-3.2	9.2	-0.6	-5.7	5.0	-9.0

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
4	D	-5.0	-6.1	0.6	-2.9	0.2	-6.1	-0.4	-5.3	-0.3	-0.2	0.3	0.2
4	A	-0.6	-2.9	5.0	-6.1	0.4	-5.3	-0.2	-6.1	-0.3	0.2	0.3	-0.2

Frame Line	Column Line	MIN_SNOW		F4UNB_SL_L		F4UNB_SL_R	
		Horz	Vert	Horz	Vert	Horz	Vert
4	D	1.4	4.1	2.5	8.5	2.5	4.9
4	A	-1.4	4.1	-2.5	4.9	-2.5	8.5

BUILDING BRACING REACTIONS

Wall Loc	Line	Col Line	± Reactions(k)				Panel Shear (lb/ft)		Note
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Seis	
L_EW	1								(h)
F_SW	A	2,3							(a)
R_EW	4								(h)
B_SW	D	3,2	4.9	2.9	1.6	1.0			

(a) Wind bent in bay
(h) Rigid frame at endwall

WIND BENT REACTIONS

Wall Loc	Line	Col Line	± Reactions				Bolt(in) Qty	Dia	Base_Plate(in)		
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert			Width	Length	Thick
F_SW	A	2	3.7	5.1	1.0	1.4	4	0.750	6.000	12.000	0.500
F_SW	A	3	3.7	5.1	1.0	1.4	4	0.750	6.000	12.000	0.500

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	40.0
Length (ft)	=	60.0
Eave Height (ft)	=	14.0/14.0
Roof Slope (rise/12)	=	3.0/3.0
Dead Load (psf)	=	2.0
Collateral Load (psf)	=	2.0
Live Load (psf)	=	20.0
Snow Load (psf)	=	45.0
Ultimate Wind Speed (Vult)	=	145.0
Wind Code	=	IBC-15
Exposure	=	C
Closed/Open	=	C
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Zone	=	B
Seismic Coeff (Fa*Sa)	=	0.41

ID Description

1	Dead+Collateral+Snow+Slide_Snow
2	Dead+Collateral+0.75Snow+0.45Wind_Left1+0.75Side_Snow
3	0.6Dead+0.6Wind_Left1
4	0.6Dead+0.6Wind_Right1
5	0.6Dead+0.6Wind_Left2
6	0.6Dead+0.6Wind_Right2
7	0.6Dead+0.6Wind_Long1L
8	0.6Dead+0.6Wind_Right2+0.6Wind_Suction
9	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
10	Dead+0.6Wind_Right2+0.6Wind_Suction

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
12	Endwall	5/8"	A307
32	Frame	3/4"	A307
8	WindCol	3/4"	A307

NOTE:
THIS DESIGN IS BASED ON RIGID FRAMES
LINE 1 & LINE 2 ARE TO SUPPORT FUTURE LEAN-TO
10' WIDE X 20' LONG X 10' LOW EAVE HT/12'-6"
HIGH EAVE HT, 3:12

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

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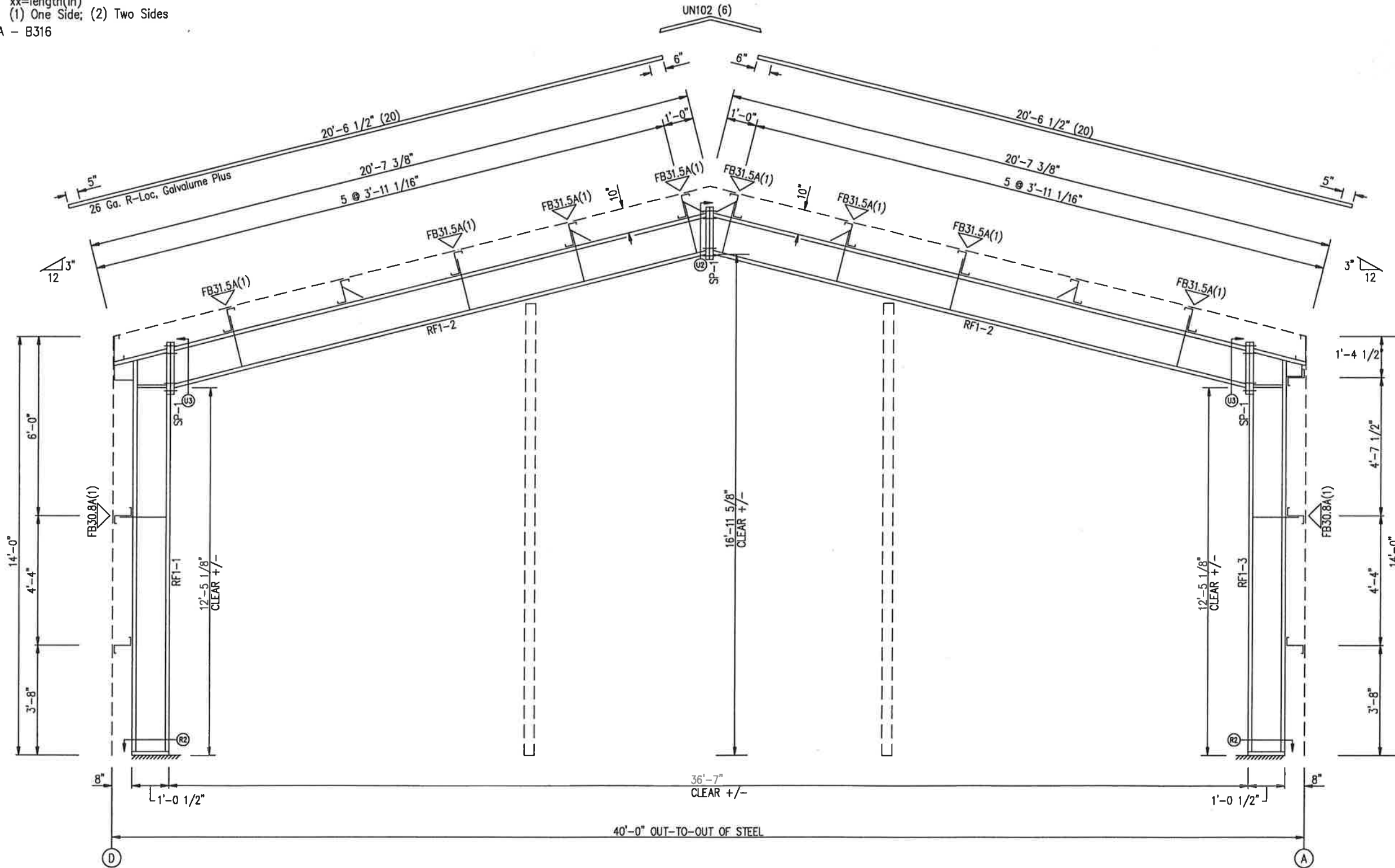
DESCRIPTION	ANCHOR BOLT REACTIONS
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE

JOB NO: 55101
ENG. BY: SS
DATE: 04/25/17
DWG. NO: 3 OF 16
ISSUE: E

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	0	A325	0.625	2.00

MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	Inside Flange
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length
RF1-1	12.0	12.0	0.135	11'-10 3/16"	5 x 1/4" x 13'-2 15/16"	5 x 1/4" x 12'-1 3/16"
RF1-2	12.0	12.0	0.188	1'-7 13/16"	5 x 1/4" x 1'-8 13/16"	5 x 1/4" x 18'-8 7/8"
RF1-3	12.0	12.0	0.135	19'-0 3/16"	5 x 1/4" x 18'-8 7/8"	5 x 1/4" x 12'-1 3/16"
	12.0	12.0	0.188	1'-7 13/16"	5 x 1/4" x 1'-8 13/16"	5 x 1/4" x 12'-1 3/16"
	12.0	12.0	0.135	11'-10 3/16"	5 x 1/4" x 13'-2 15/16"	5 x 1/4" x 12'-1 3/16"

▽ FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 A - B316



NOTE:
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 LINE 1 & LINE 2 ARE TO SUPPORT FUTURE LEAN-TO
 10' WIDE X 20' LONG X 10' LOW EAVE HT/12'-6"
 HIGH EAVE HT, 3:12

RIGID FRAME ELEVATION: FRAME LINE 1

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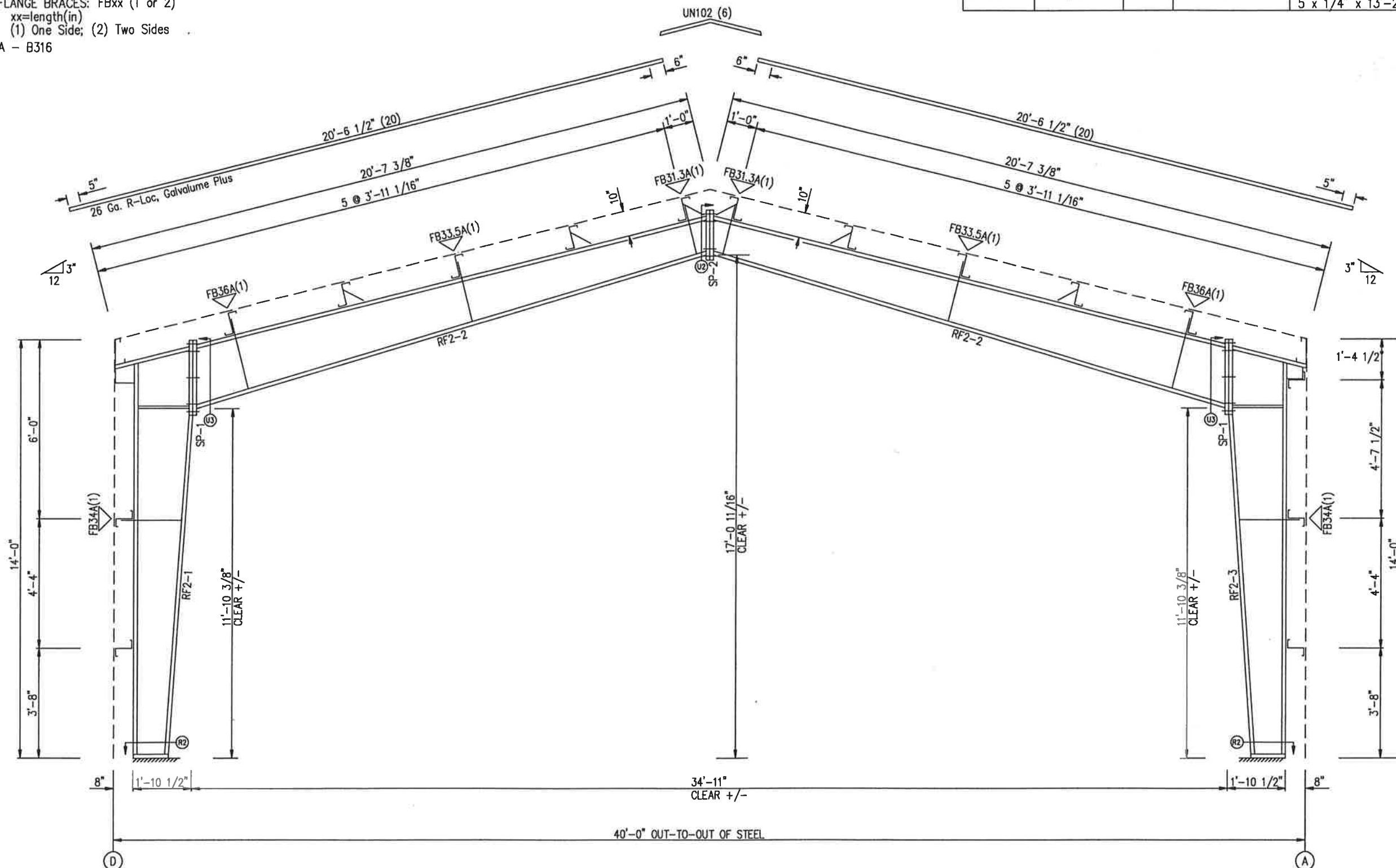


DESCRIPTION	RIGID FRAME ELEVATION		
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC		
END USER	CHRIS BERTOLINO		
SCALE	NOT TO SCALE		
JOB NO.	55101	ENG. BY:	SS
DATE:	04/25/17	DWG. NO.:	4 OF 16
ISSUE:	E		

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	2	A325	0.625	2.25
SP-2	4	4	0	A325	0.625	2.00

MEMBER TABLE								
Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length	W x Thk x Length	
RF2-1	11.0	22.0	0.188	13'-8 1/2"	5 x 1/4" x 13'-2 15/16"	5 x 1/4" x 11'-6 13/16"		
RF2-2	22.0	17.3	0.188	8'-4 7/8"	5 x 1/4" x 2'-7 1/8"	5 x 1/4" x 18'-1 1/16"		
	17.3	12.0	0.135	9'-11"	5 x 1/4" x 17'-10 5/16"			
RF2-3	22.0	11.0	0.188	13'-8 1/2"	5 x 1/4" x 2'-7 1/8"	5 x 1/4" x 11'-6 13/16"		

▽ FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 A - B316



NOTE:
 THIS DESIGN IS BASED ON RIGID FRAMES @
 LINE 1 & LINE 2 ARE TO SUPPORT FUTURE LEAN-TO
 10' WIDE X 20' LONG X 10' LOW EAVE HT/12'-6"
 HIGH EAVE HT, 3:12

RIGID FRAME ELEVATION: FRAME LINE 2

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ARMSTRONG
STEEL

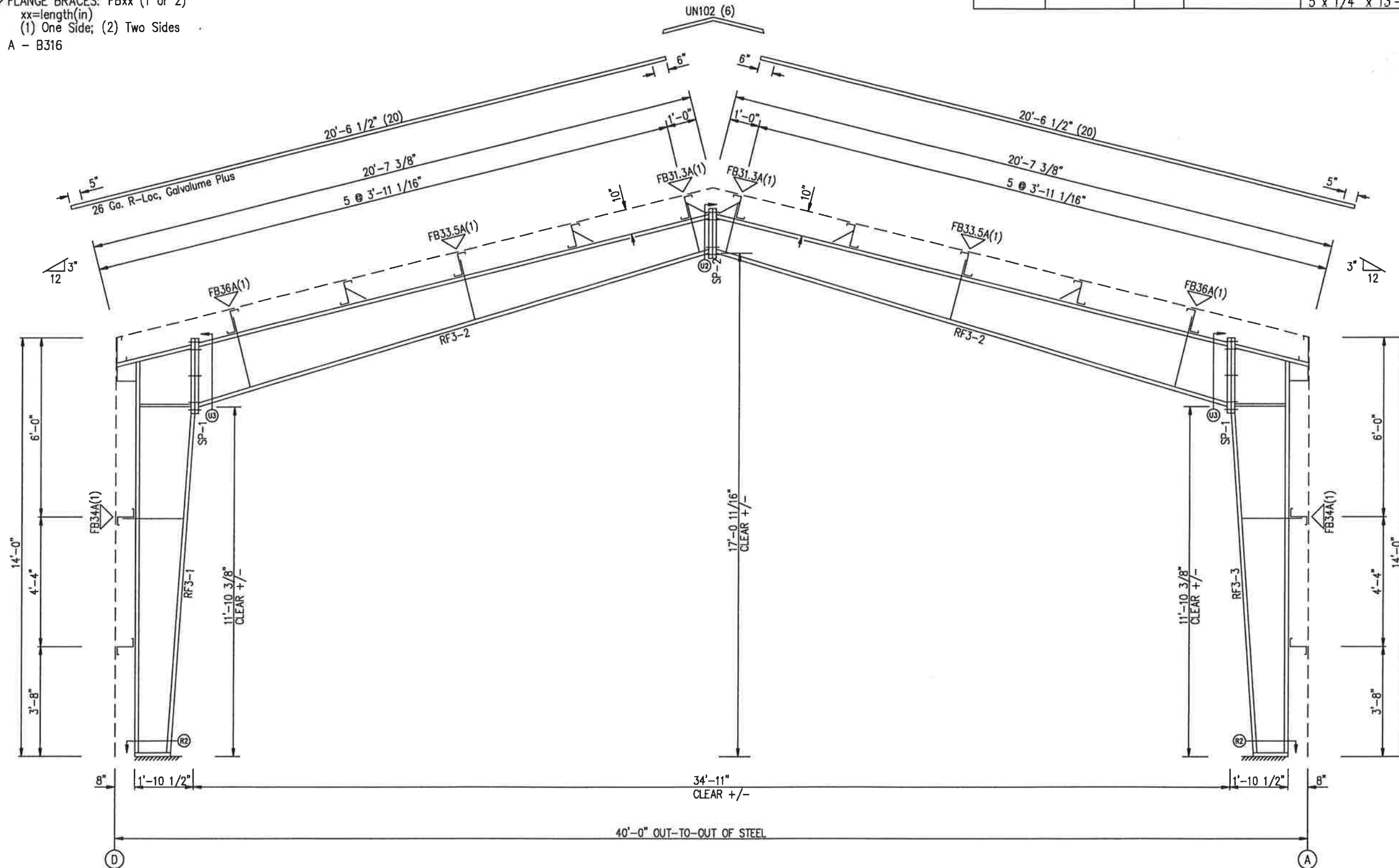
5889 S. Greenwood Plaza Blvd, Ste #300
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DESCRIPTION	RIGID FRAME ELEVATION
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE
JOB NO.:	55101
ENCL. BY:	SS
DATE:	04/25/17
DWG. NO.:	5 OF 16
ISSUE:	E

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	2	A325	0.625	2.25
SP-2	4	4	0	A325	0.625	2.00

MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	Inside Flange
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length
RF3-1	11.0	22.0	0.188	13'-8 1/2"	5 x 1/4" x 13'-2 15/16"	5 x 1/4" x 11'-6 13/16"
RF3-2	22.0	17.3	0.188	8'-4 7/8"	5 x 1/4" x 2'-7 1/8"	5 x 1/4" x 18'-1 1/16"
RF3-3	22.0	11.0	0.188	13'-8 1/2"	5 x 1/4" x 2'-7 1/8"	5 x 1/4" x 11'-6 13/16"

▽ FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 A - B316



NOTE:
 THIS DESIGN IS BASED ON RIGID FRAMES @
 LINE 1 & LINE 2 ARE TO SUPPORT FUTURE LEAN-TO
 10' WIDE X 20' LONG X 10' LOW EAVE HT/12'-6"
 HIGH EAVE HT, 3:12

RIGID FRAME ELEVATION: FRAME LINE 3

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY
 AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
 ENSURE PROPER FIT. SUCH WORK IS CONSIDERED
 A NORMAL PART OF METAL BUILDING ERECTION.
 A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR
 FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS



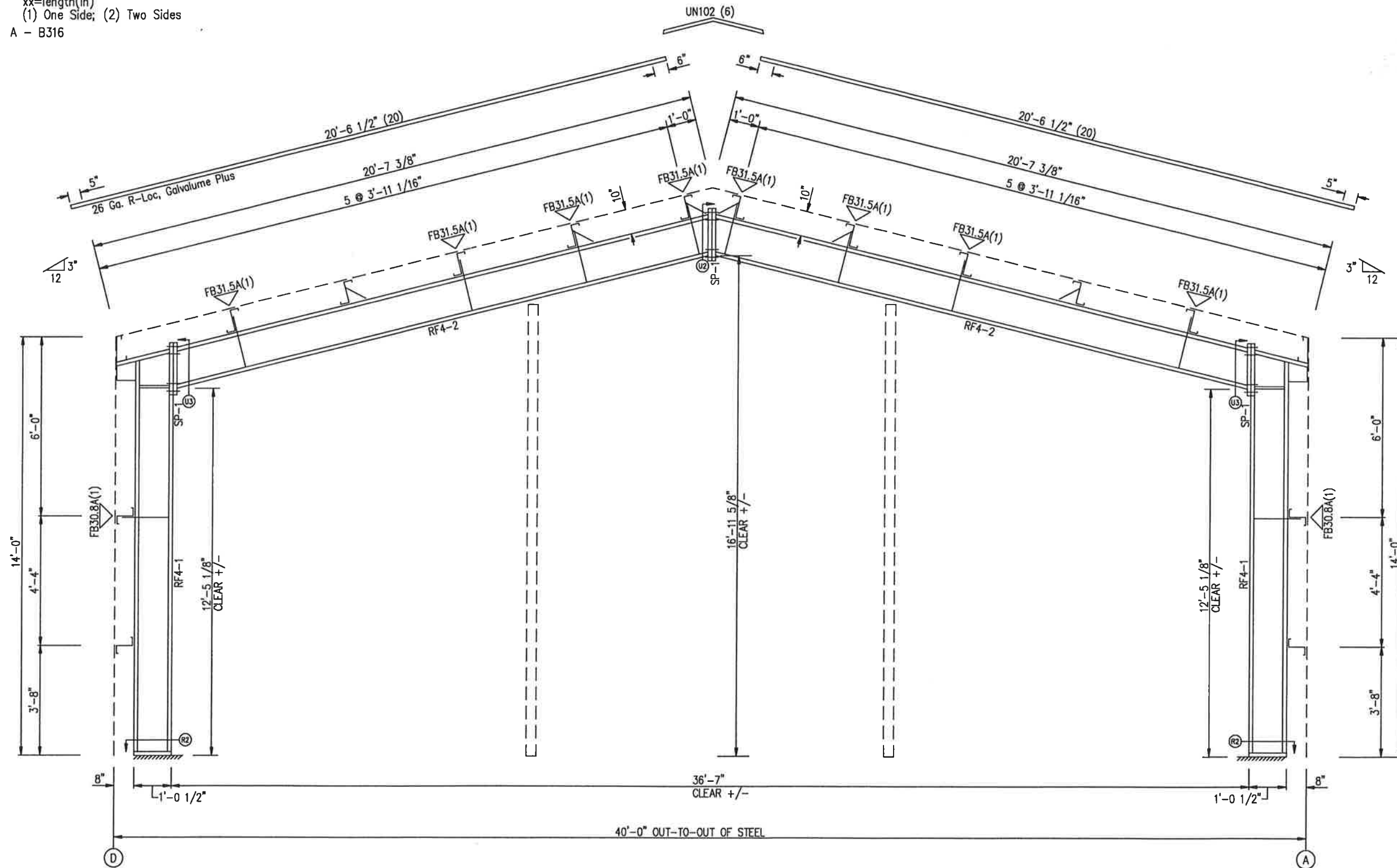
5589 S. Greenwood Plaza Blvd, Ste#300
 Greenwood Village, Colorado 80111
 PHONE: 800-345-4810
 www.armstrongsteel.com

DESCRIPTION	RIGID FRAME ELEVATION
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE
JOB NO.:	55101
ENG. BY:	SS
DATE:	04/25/17
DWG. NO.:	6 OF 16
ISSUE:	E

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	0	A325	0.625	2.00

MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	Inside Flange
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length
RF4-1	12.0	12.0	0.135	11'-10 3/16"	5 x 1/4" x 13'-2 15/16"	5 x 1/4" x 12'-1 3/16"
RF4-2	12.0	12.0	0.188	1'-7 13/16"	5 x 1/4" x 1'-8 13/16"	5 x 1/4" x 18'-8 7/8"
RF4-2	13.0	13.0	0.135	19'-0 3/16"	5 x 1/4" x 18'-8 7/8"	5 x 1/4" x 18'-8 7/8"

▽ FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 A - B316



NOTE:
 THIS DESIGN IS BASED ON RIGID FRAMES @
 LINE 1 & LINE 2 ARE TO SUPPORT FUTURE LEAN-TO
 10' WIDE X 20' LONG X 10' LOW EAVE HT/12'-6"
 HIGH EAVE HT, 3:12

NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY
 AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
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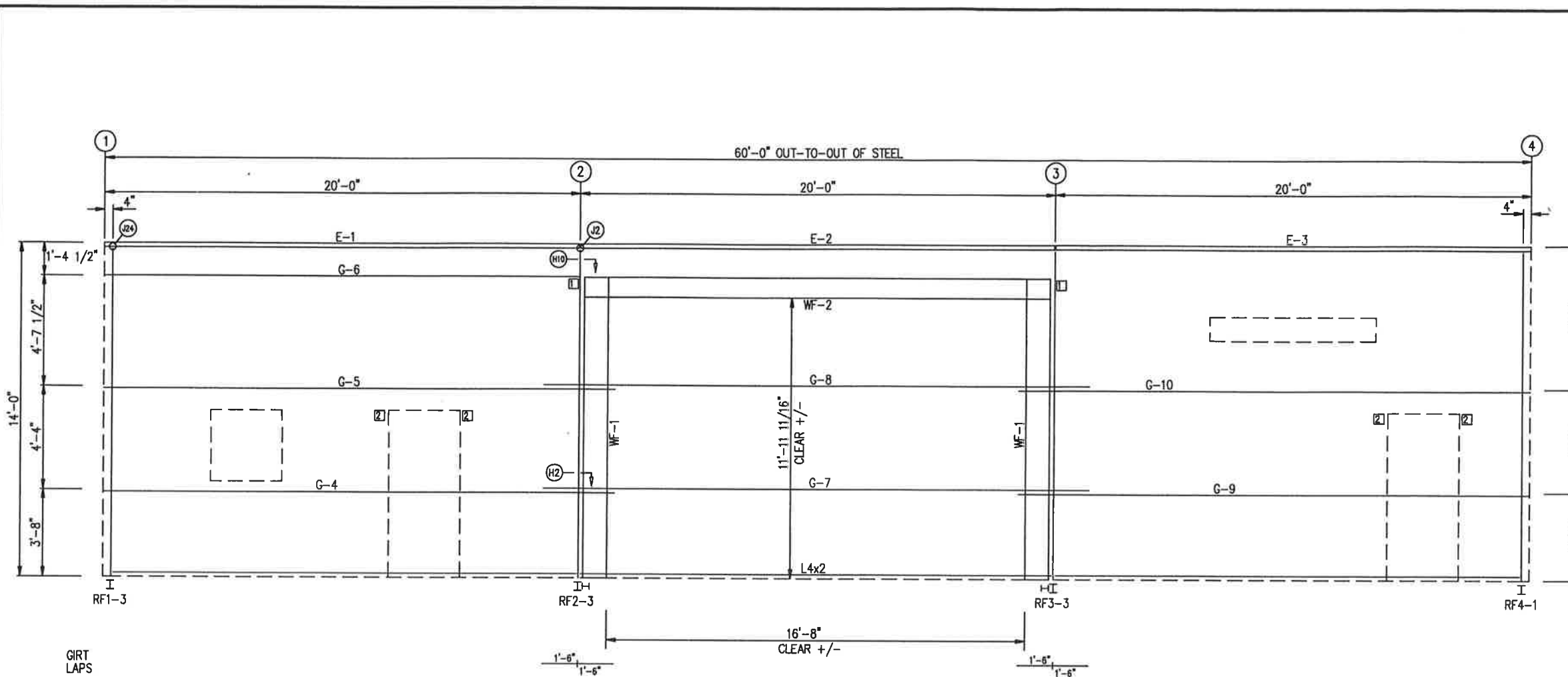
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

ARMSTRONG
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 Greenwood Village, Colorado 80111
 PHONE: 800-345-4610
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DESCRIPTION	RIGID FRAME ELEVATION
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE

JOB NO.: 55101
 ENG. BY: SS
 DATE: 04/25/17
 DWG. NO.: 7 OF 16
 ISSUE: E



SIDEWALL FRAMING: FRAME LINE A

TRIM TABLE FRAME LINE A				
ID	QUAN	PART	LENGTH	DETAIL
1	3	BA204	20'-4"	TRIM_1
2	2	OU142	14'-2"	TRIM_30
3	3	Q772204	20'-4"	TRIM_61
4	1	Q773L	6"	TRIM_60
5	1	AR961L	7 7/16"	TRIM_60
6	1	Q773R	6"	TRIM_60
7	1	AR961R	7 7/16"	TRIM_60
8	3	HE036	3'-6"	TRIM_51
9	4	JA072	7'-2"	TRIM_50
10	1	HE073	7'-3"	TRIM_51

BOLT TABLE FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	5/8"	2"
WF-1 - RF2-3	8	A325	5/8"	1 1/2"
WF-1 - RF3-3	8	A325	5/8"	1 1/2"

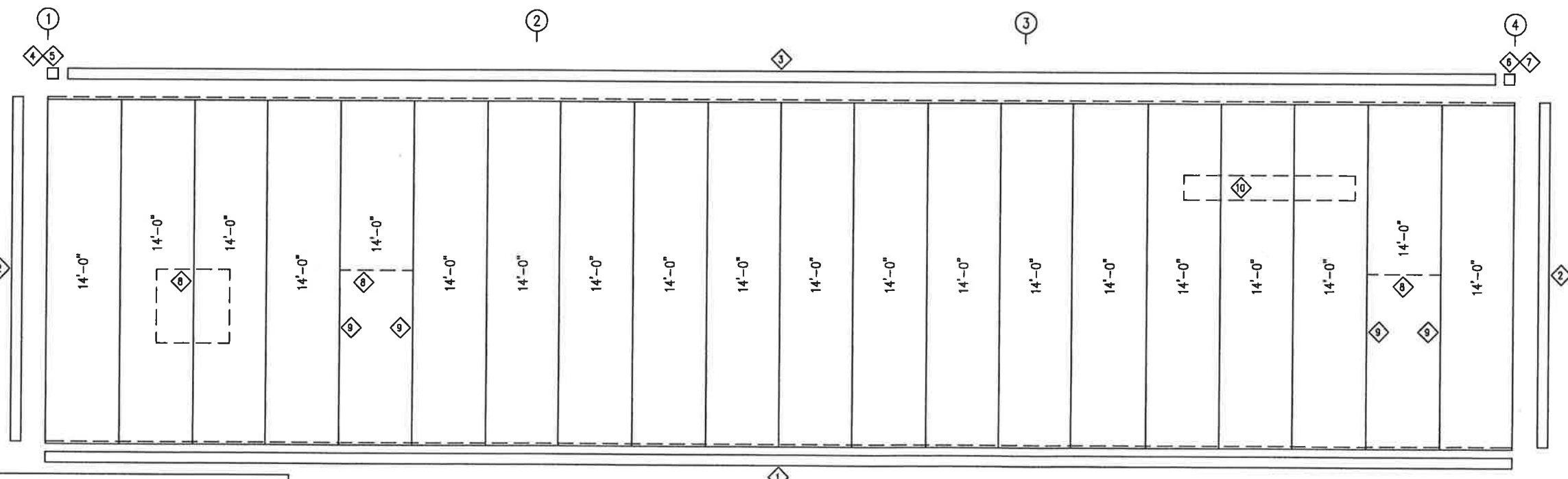
MEMBER TABLE FRAME LINE A				
QUAN	MARK	PART	LENGTH	
2	WF-1	W12531	12'-9 11/16"	
1	WF-2	W10631	16'-7 3/8"	
1	E-1	10376DU3	19'-11"	
1	E-2	10376DU3	19'-11"	
1	E-3	10376DU3	19'-11"	
1	G-4	8X25Z16	21'-5 1/2"	
1	G-5	8X25Z12	21'-5 1/2"	
1	G-6	8X35Z14	19'-10 3/4"	
1	G-7	8X25Z16	23'-0"	
1	G-8	8X25Z14	23'-0"	
1	G-9	8X25Z16	21'-5 1/2"	
1	G-10	8X25Z12	21'-5 1/2"	

CONNECTION PLATES FRAME LINE A			
ID	QUAN	MARK/PART	
1	2	BC-49	
2	4	BC-500	

NOTE:
FIELD CUT GIRT/PANEL & DRILL 9/16"Ø HOLES FOR FIELD LOCATED WALK DOOR FRAMED OPENING.

NOTE:
INSTALL WINDOW(S) FIRST BEFORE INSTALLING WINDOW FRAMING (IF NEEDED)-ACC_11

NOTE:
FIELD CUT PANEL FOR FIELD LOCATED SELF FRAMING/SELF FLASHING WINDOWS.



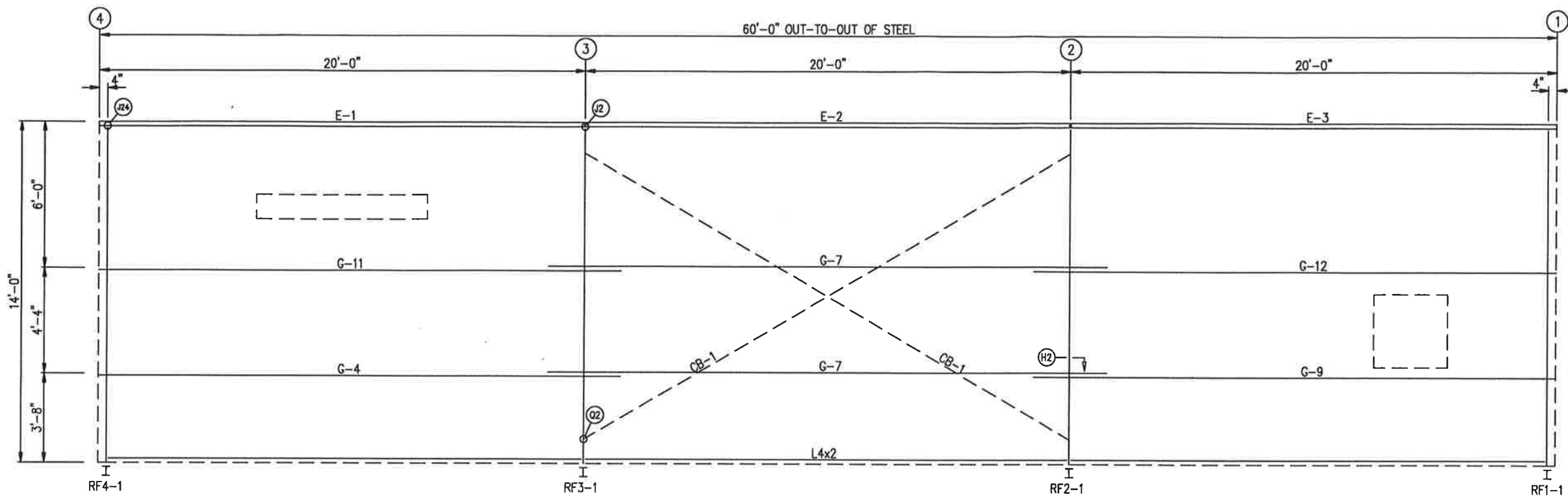
SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 Ga. R-Loc - Desert 40 yr

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

5889 S. Greenwood Plaza Blvd, Ste#300
Greenwood Village, Colorado 80111
PHONE: 800-345-4810
www.armstrongsteel.com

DESCRIPTION	SIDEWALL FRAMING & SHEETING
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE
ISSUE NO.	55101
DATE	04/25/17
ISSUE	E



GIRT LAPS

1'-6" 1'-6" 1'-6" 1'-6"

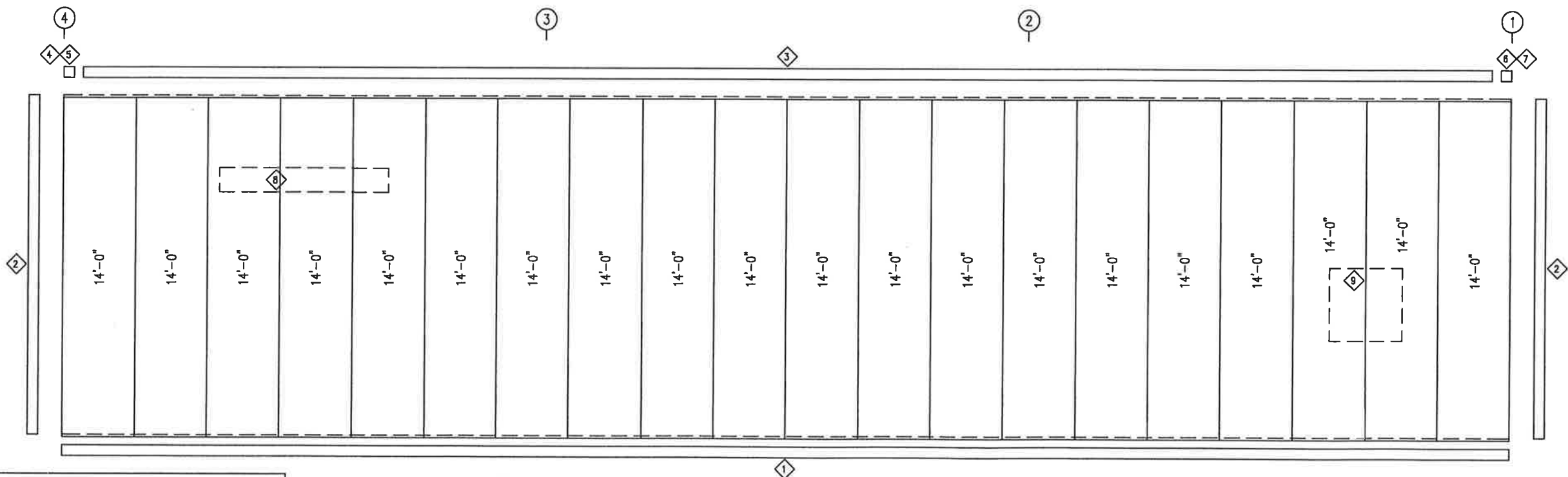
SIDEWALL FRAMING: FRAME LINE D

TRIM TABLE FRAME LINE D				
ID	QUAN	PART	LENGTH	DETAIL
1	3	BA204	20'-4"	TRIM_1
2	2	OU142	14'-2"	TRIM_30
3	3	Q772204	20'-4"	TRIM_61
4	1	Q773L	6"	TRIM_60
5	1	AR961L	7 7/16"	TRIM_60
6	1	Q773R	6"	TRIM_60
7	1	AR961R	7 7/16"	TRIM_60
8	1	HE073	7'-3"	TRIM_51
9	1	HE036	3'-6"	TRIM_51

MEMBER TABLE FRAME LINE D			
QUAN	MARK	PART	LENGTH
1	E-1	10376DU3	19'-11"
1	E-2	10376DU3	19'-11"
1	E-3	10376DU3	19'-11"
1	G-4	8X25Z16	21'-5 1/2"
2	G-7	8X25Z16	23'-0"
1	G-9	8X25Z16	21'-5 1/2"
1	G-11	8X25Z14	21'-5 1/2"
1	G-12	8X25Z14	21'-5 1/2"
2	CB-1	GS1720	23'-9 1/4"

NOTE:
INSTALL WINDOW(S) FIRST BEFORE INSTALLING WINDOW FRAMING (IF NEEDED)-ACC_11

NOTE:
FIELD CUT PANEL FOR FIELD LOCATED SELF FRAMING/SELF FLASHING WINDOWS.



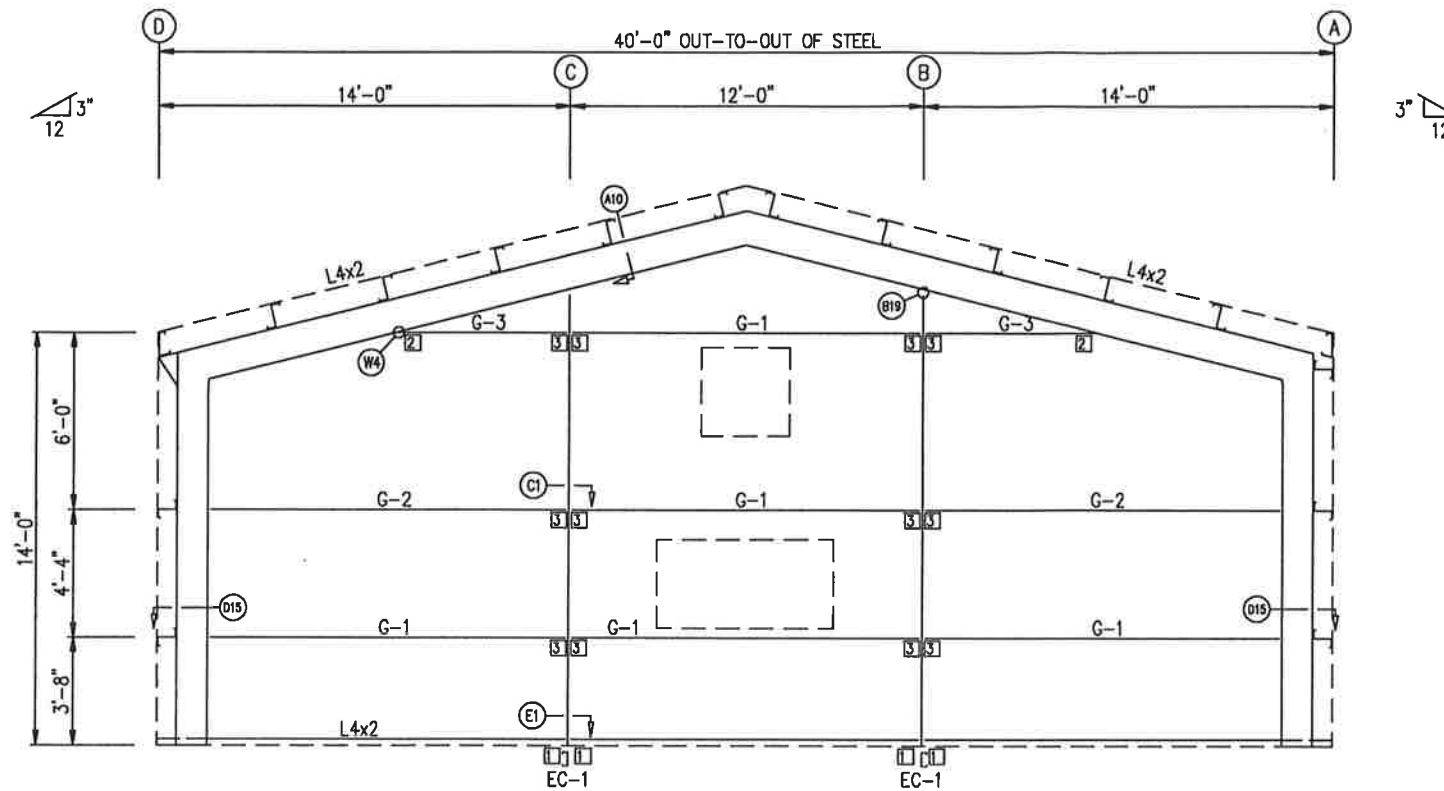
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SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 Ga. R-Loc - Desert 40 yr

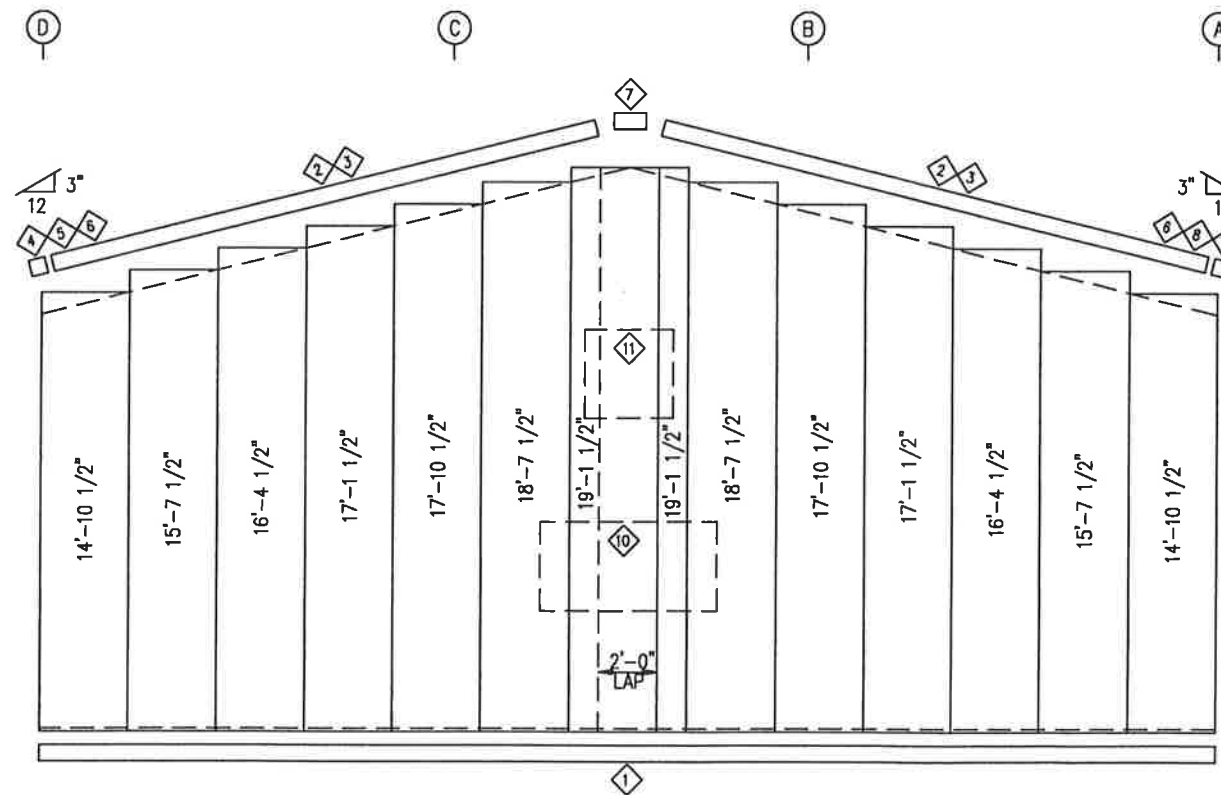
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

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Greenwood Village, Colorado 80111
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	SIDEWALL FRAMING & SHEETING	
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC	
END USER	CHRIS BERTOLINO	
SCALE	NOT TO SCALE	
JOB NO.:	55101	DATE: 04/25/17
ISSUE:	SS	ISSUE: E
DWG. NO.:	9 OF 16	



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. R-Loc - Desert 40 yr

TRIM TABLE FRAME LINE 1				
ID	QUAN	PART	LENGTH	DETAIL
1	2	BA204	20'-4"	TRIM_1
2	2	Q764102	10'-2"	TRIM_66
3	2	Q764122	12'-2"	TRIM_66
4	1	Q765L	6"	TRIM_60
5	1	AR963L	9 1/8"	TRIM_60
6	2	AR962	8 1/16"	TRIM_60
7	1	Q767	1'-4"	TRIM_100
8	1	Q765R	6"	TRIM_60
9	1	AR963R	9 1/8"	TRIM_60
10	1	HE063	6'-3"	TRIM_51
11	1	HE036	3'-6"	TRIM_51

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	1/2"	1 1/4"

MEMBER TABLE FRAME LINE 1			
QUAN	MARK	PART	LENGTH
2	EC-1	8X35C12	15'-1 3/4"
5	G-1	8X25Z16	11'-11"
2	G-2	8X25Z14	11'-11"
2	G-3	8X25Z16	5'-3 1/8"

CONNECTION PLATES FRAME LINE 1		
ID	QUAN	MARK/PART
1	4	BC-04
2	2	BC-15D
3	12	BC-01

NOTE:
INSTALL WINDOW(S) FIRST BEFORE INSTALLING WINDOW FRAMING (IF NEEDED)-ACC_11

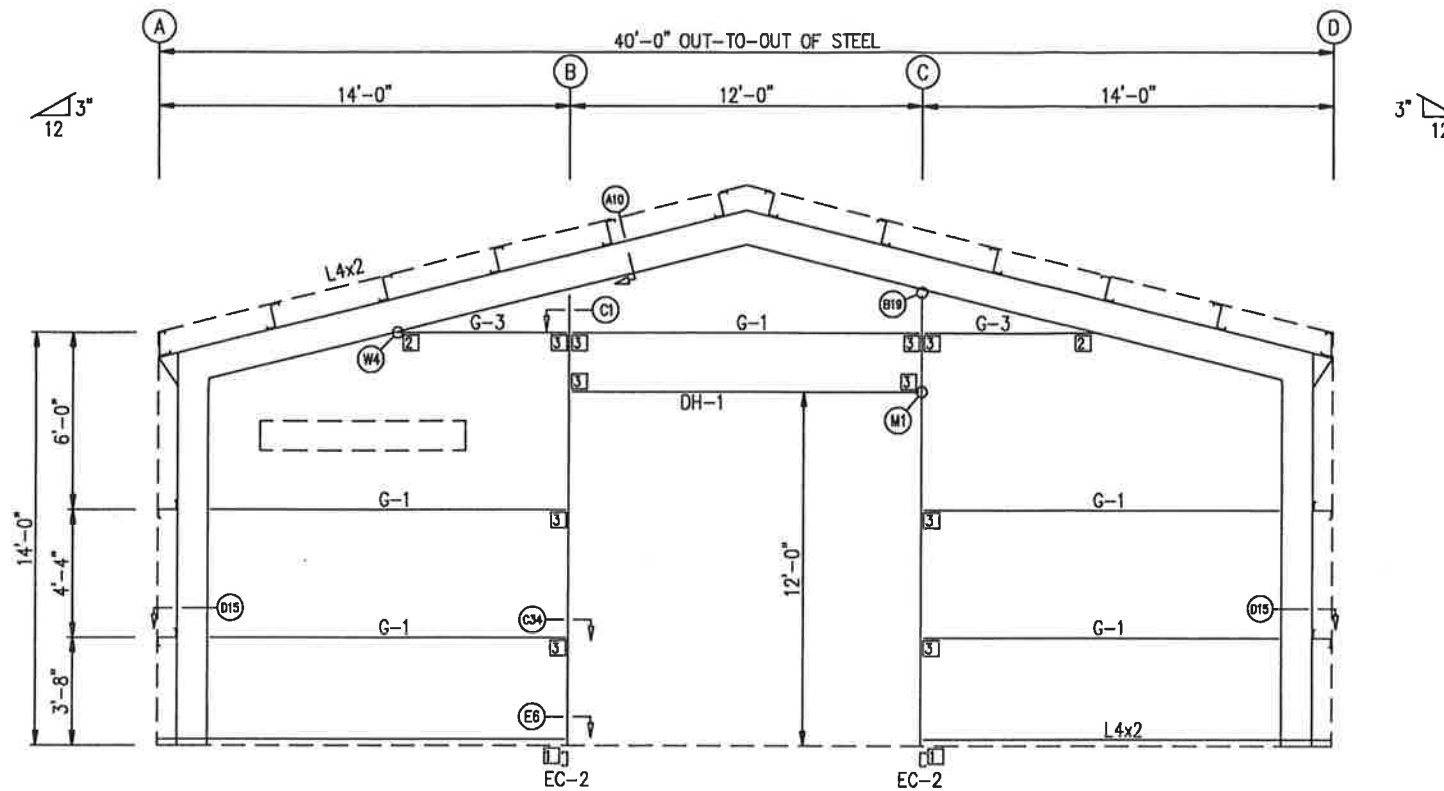
NOTE:
FIELD CUT PANEL FOR FIELD LOCATED SELF FRAMING/SELF FLASHING WINDOWS.

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

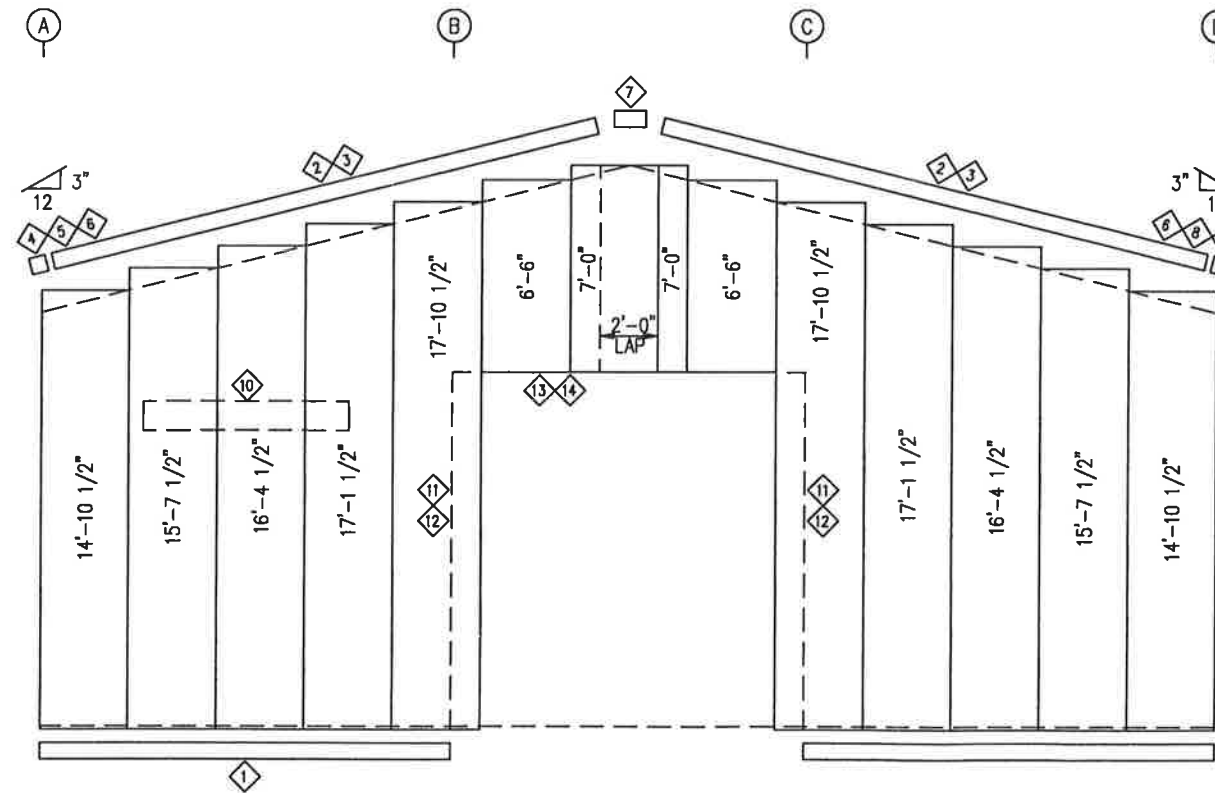
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

5889 S. Greenwood Plaza Blvd, Ste#300
Greenwood Village, Colorado 80111
PHONE: 800-345-4810
www.armstrongsteel.com

DESCRIPTION		ENDWALL FRAMING & SHEETING	
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC		
END USER	CHRIS BERTOLINO		
SCALE	NOT TO SCALE		
JOB NO:	55101	ENG. BY:	SS
DWG. NO.:	10 OF 16	DATE:	04/25/17
		ISSUE:	E



ENDWALL FRAMING: FRAME LINE 4



ENDWALL SHEETING & TRIM: FRAME LINE 4

PANELS: 26 Ga. R-Loc - Desert 40 yr

TRIM TABLE FRAME LINE 4				
ID	QUAN	PART	LENGTH	DETAIL
1	2	BA204	20'-4"	TRIM_1
2	2	Q764102	10'-2"	TRIM_66
3	2	Q764122	12'-2"	TRIM_66
4	1	Q765L	6"	TRIM_60
5	1	AR963L	9 1/8"	TRIM_60
6	2	AR962	8 1/16"	TRIM_60
7	1	Q767	1'-4"	TRIM_100
8	1	Q765R	6"	TRIM_60
9	1	AR963R	9 1/8"	TRIM_60
10	1	HE073	7'-3"	TRIM_51
11	2	Q370142	14'-2"	TRIM_50
12	2	JA122	12'-2"	TRIM_50
13	1	Q370122	12'-2"	TRIM_51
14	1	HE123	12'-3"	TRIM_51

BOLT TABLE FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	1/2"	1 1/4"

MEMBER TABLE FRAME LINE 4			
QUAN	MARK	PART	LENGTH
2	EC-2	8X35C12	15'-1 3/4"
1	DH-1	8X25C16	11'-11"
5	G-1	8X25Z16	11'-11"
2	G-3	8X25Z16	5'-3 1/8"

CONNECTION PLATES FRAME LINE 4		
ID	QUAN	MARK/PART
1	2	BC-04
2	2	BC-15D
3	10	BC-01

NOTE:
INSTALL WINDOW(S) FIRST BEFORE INSTALLING WINDOW FRAMING (IF NEEDED)-ACC_11

NOTE:
FIELD CUT PANEL FOR FIELD LOCATED SELF FRAMING/SELF FLASHING WINDOWS.

NOTE:
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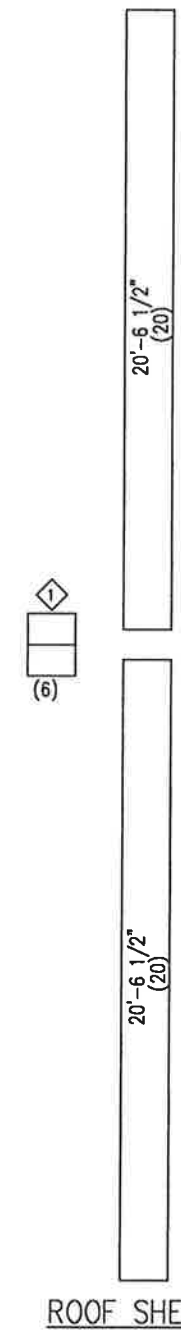
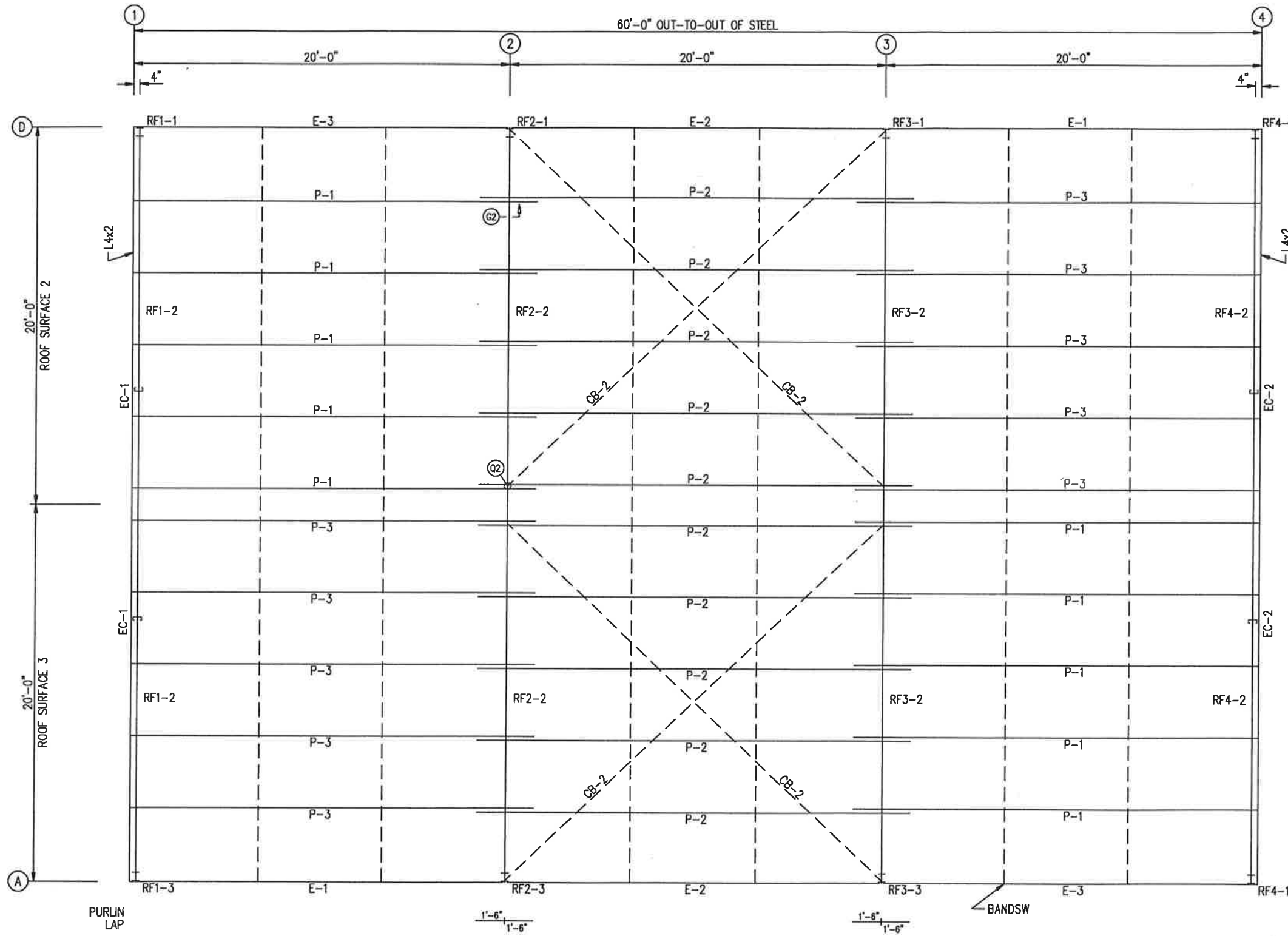
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

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Greenwood Village, Colorado 80111
PHONE: 800-345-4810
www.armstrongsteel.com

DESCRIPTION	ENDWALL FRAMING & SHEETING
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE
JOB NO:	55101
ENGL BY:	SS
DATE:	04/25/17
DWG NO.:	11 OF 16
ISSUE:	E

TRIM TABLE				
ROOF PLAN				
ID	QUAN	PART	LENGTH	DETAIL
1	6	UN102	10'-2"	TRIM_101

MEMBER TABLE				
ROOF PLAN				
QUAN	MARK	PART	LENGTH	
10	P-1	10x35Z14	21'-5 1/2"	
10	P-2	10x35Z14	23'-0"	
10	P-3	10x35Z14	21'-5 1/2"	
2	E-1	10376DU3	19'-11"	
2	E-2	10376DU3	19'-11"	
2	E-3	10376DU3	19'-11"	
4	CB-2	GS1716	27'-4 1/4"	



ROOF FRAMING PLAN

ROOF SHEETING
PANELS: 26 Ga. R-Loc Galvalume Plus

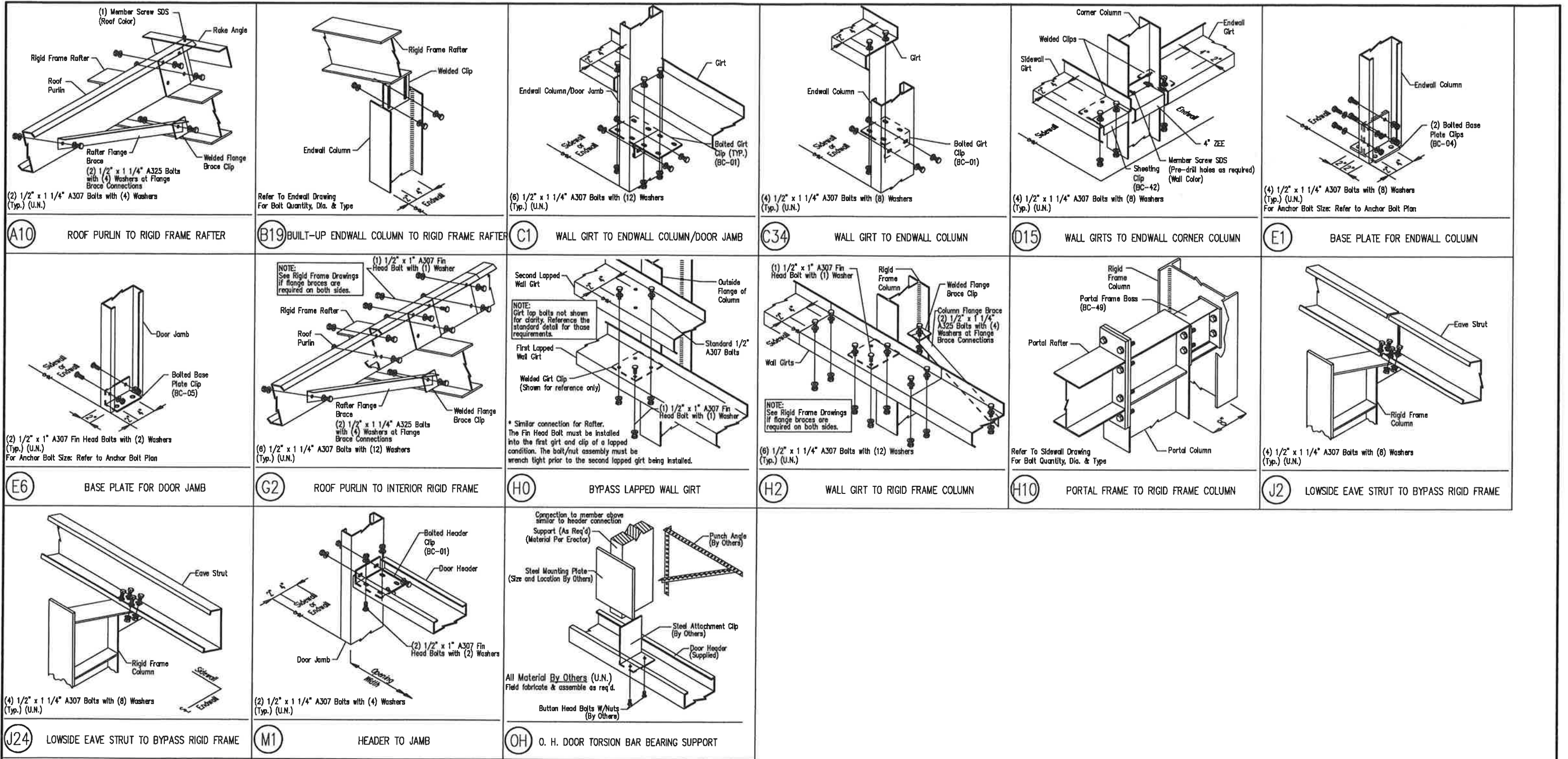
NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

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Greenwood Village, Colorado 80111
PHONE: 800-345-4610
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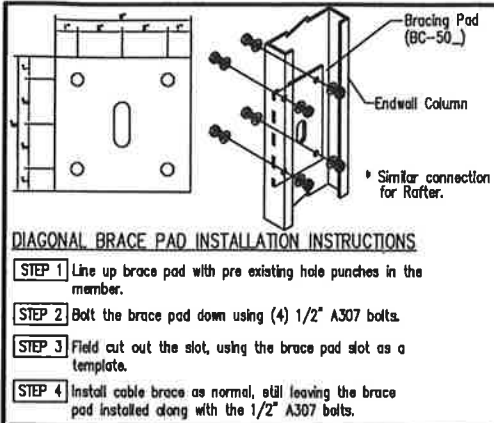
DESCRIPTION	ROOF FRAMING & SHEETING
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE
JOB NO.:	55101
ENG. BY:	SS
DWG. NO.:	12 OF 16
DATE:	04/25/17
ISSUE:	E



ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS

5889 S. Greenwood Plaza Blvd, Ste#300
 Greenwood Village, Colorado 80111
 PHONE: 800-345-4810
 www.armstrongsteel.com

DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE
JOB NO.: 55101	ENGR. BY: SS
DWG. NO.: 13 OF 16	DATE: 04/25/17
	ISSUE: E



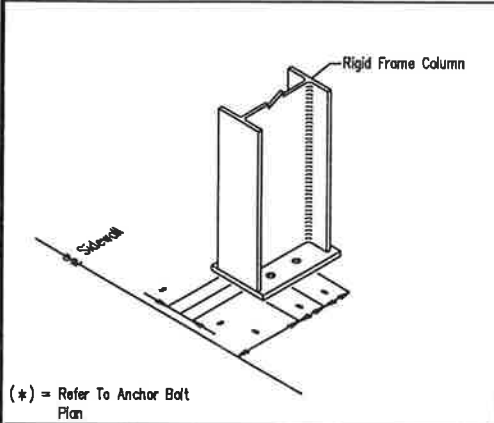
DIAGONAL BRACE PAD INSTALLATION INSTRUCTIONS

STEP 1 Line up brace pad with pre existing hole punches in the member.

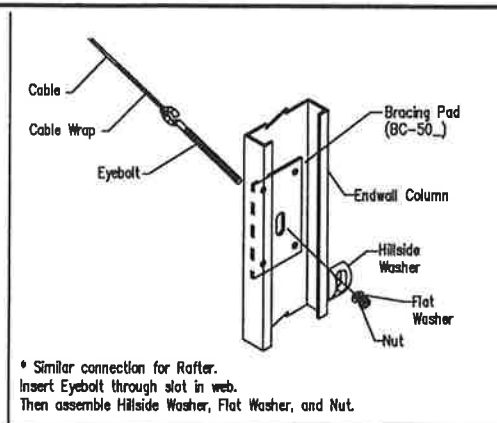
STEP 2 Bolt the brace pad down using (4) 1/2" A307 bolts.

STEP 3 Field cut out the slot, using the brace pad slot as a template.

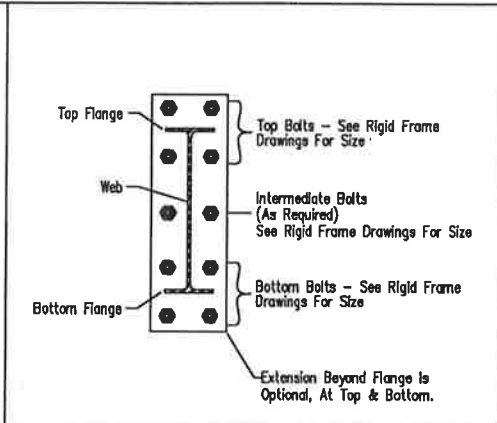
STEP 4 Install cable brace as normal, still leaving the brace pad installed along with the 1/2" A307 bolts.



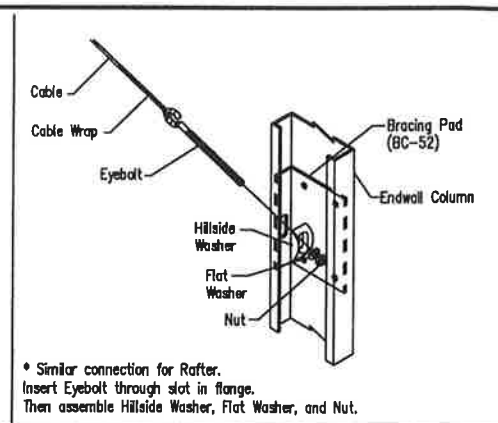
Q2 DIAGONAL BRACE PAD TO WEB OF CEE COLUMN



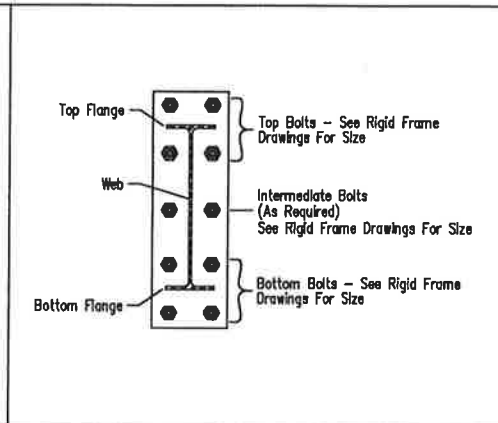
Q3 DIAGONAL CABLE BRACE TO FLANGE OF CEE COLUMN



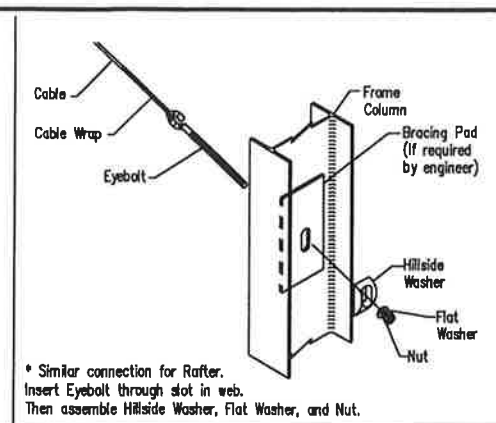
Q4 DIAGONAL CABLE BRACE TO WEB OF FRAME COLUMN



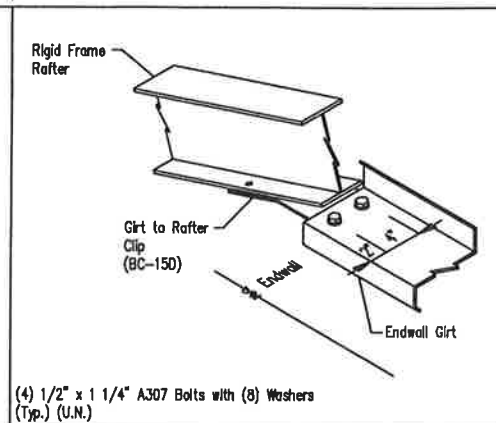
Q5 DIAGONAL CABLE BRACE TO FLANGE OF FRAME COLUMN



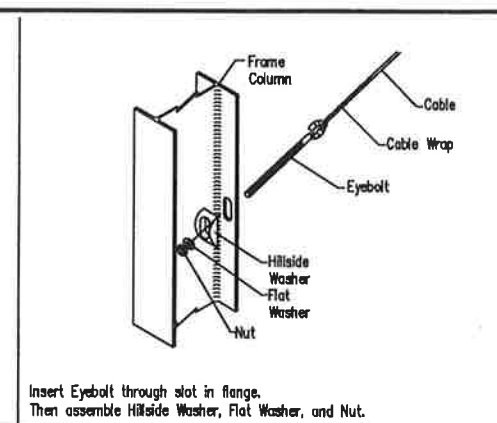
Q6 DIAGONAL CABLE BRACE AT FLUSH WALL GIRTS



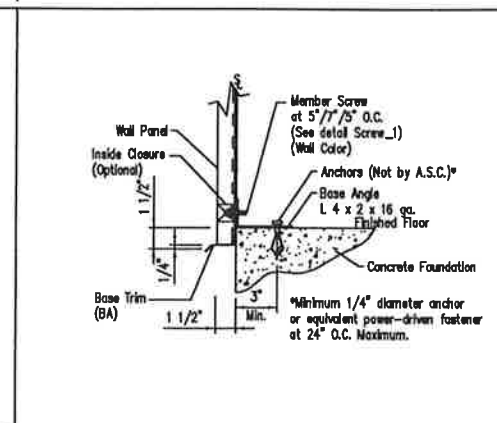
R2 ANCHOR BOLTS AT SIDEWALL COLUMNS



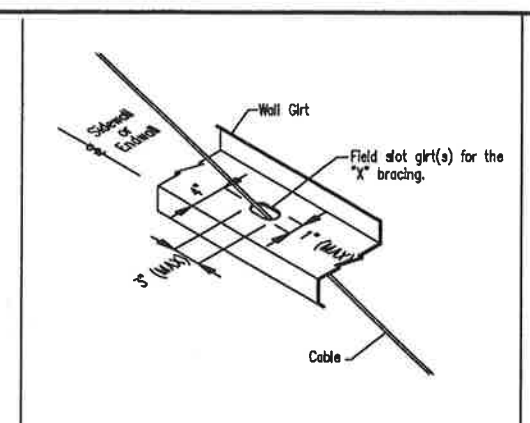
U2 BOLTS FOR RIGID FRAME RAFTER AT BUILDING PEAK



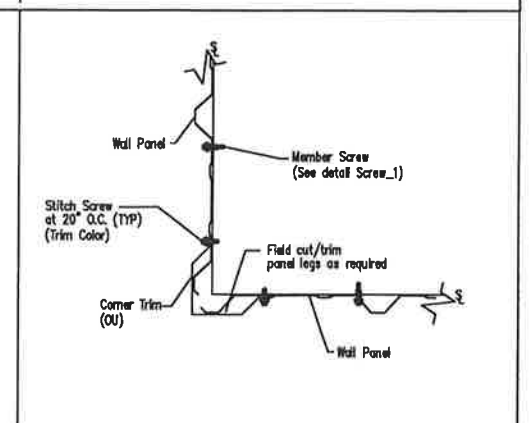
U3 BOLTS FOR RIGID FRAME RAFTER TO COLUMN CONNECTION



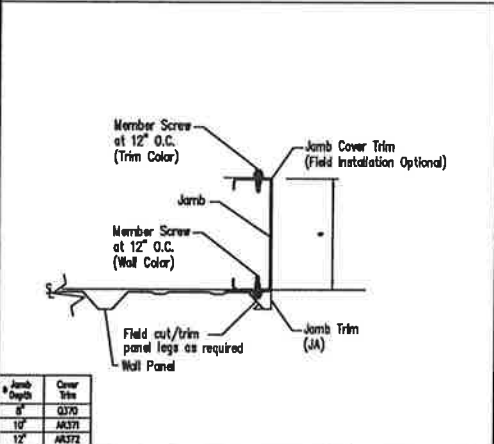
U4 ENDWALL GIRTS TO RIGID FRAME RAFTER



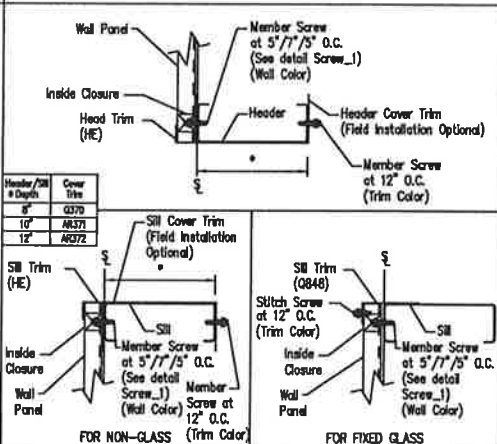
U5 FRAMED OPENING HEAD & SILL TRIM DETAILS



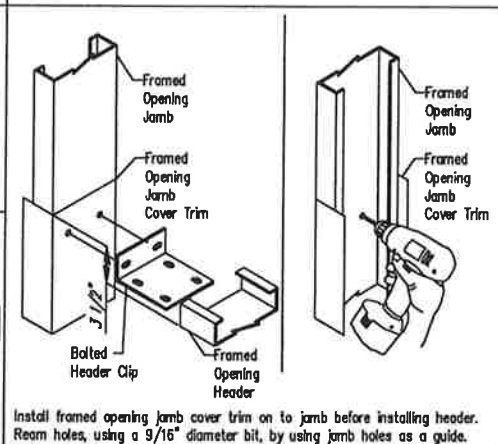
W4 EAVE DETAIL WITH STYLE EAVE AT SHEETED WALL



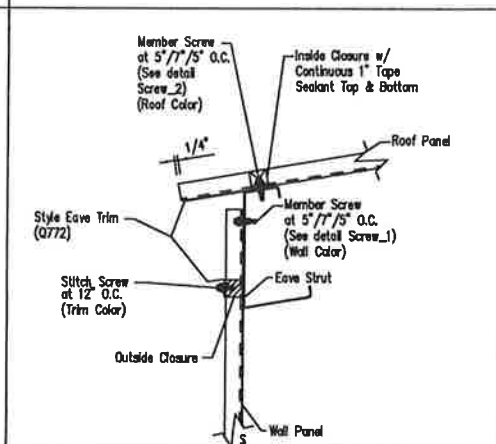
TRIM_50



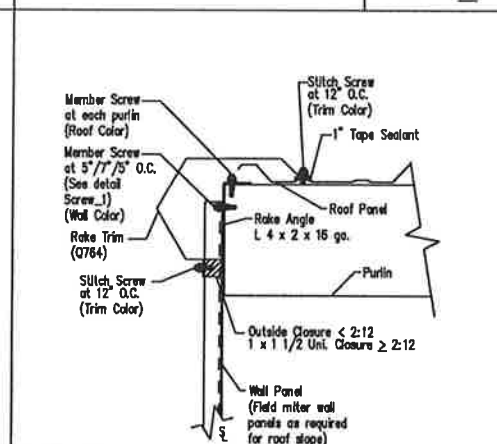
TRIM_51



TRIM_52



TRIM_61



TRIM_66

Jamb Depth	Cover Trim
8"	0370
10"	AK371
12"	AK372

Header/Sill #Depth	Cover Trim
8"	0370
10"	AK371
12"	AK372

Header/Sill #Depth	Cover Trim
8"	0370
10"	AK371
12"	AK372

Header/Sill #Depth	Cover Trim
8"	0370
10"	AK371
12"	AK372

Header/Sill #Depth	Cover Trim
8"	0370
10"	AK371
12"	AK372

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT/CONSTRUCTION	06.08.17	CAF	RS	SS
E	ERECTION	06.14.17	DK	DK	SS



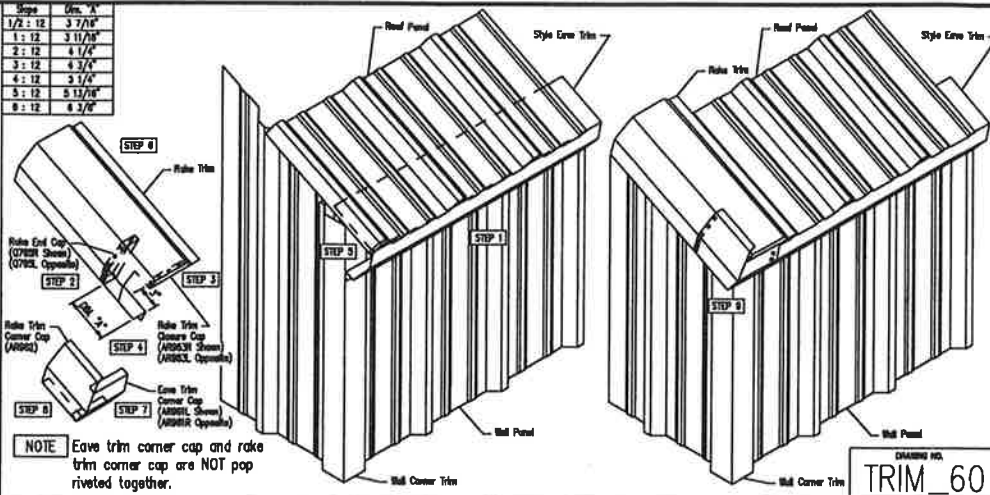
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DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	CHRIS BERTOLINO AND/OR POST SCRIPTS LLC
END USER	CHRIS BERTOLINO
SCALE	NOT TO SCALE
JOB NO.	55101
ENGR. BY:	SS
DATE:	04/25/17
DWG. NO.:	14 OF 16
ISSUE:	E

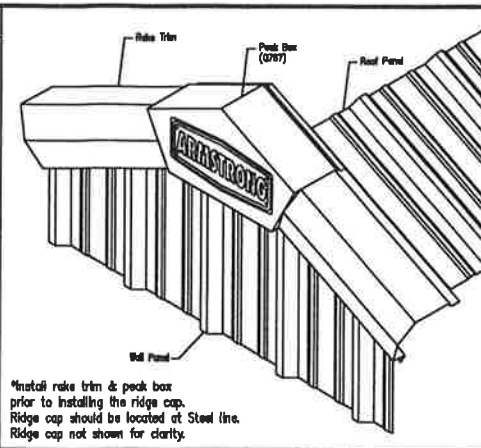
STYLE EAVE CORNER TRIM INSTALLATION INSTRUCTIONS

- STEP 1** Install style eave trim in between the roof panel and low eave member. Be sure the end of the style eave trim is flush with the wall corner trim.
- STEP 2** Install rake end cap, into rake trim using (8) pop rivets. Use chart to determine how far the rake end cap is positioned into the rake trim.
- STEP 3** Install rake trim closure cap, flush with the end of the rake trim using (5) pop rivets.
- STEP 4** Field cut/notch the face of the rake trim by 3". This is to prevent the rake trim from sticking out past the style eave trim upon final assembly.
- STEP 5** Field cut/notch the end of the roof panel back 1". This is to allow the rake trim closure cap from hitting the roof panel.
- STEP 6** Install rake trim. Be sure the end of the rake trim is flush with style eave trim.
- STEP 7** Install the eave trim corner cap to the style eave trim using (6) pop rivets.
- STEP 8** Install the rake trim corner cap to the rake trim using (7) pop rivets.
- STEP 9** Field cut/notch the bottom legs of the rake trim. Horizontal leg flush with the eave trim corner cap. Vertical leg flush with the wall corner trim.

Span	Dist. "A"
1/2 : 12	3 7/16"
1 : 12	3 11/16"
2 : 12	4 1/4"
3 : 12	4 7/8"
4 : 12	5 3/4"
5 : 12	5 13/16"
9 : 12	6 3/8"

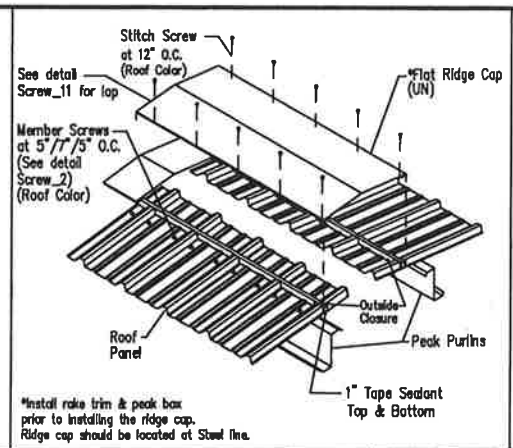


TRIM_60



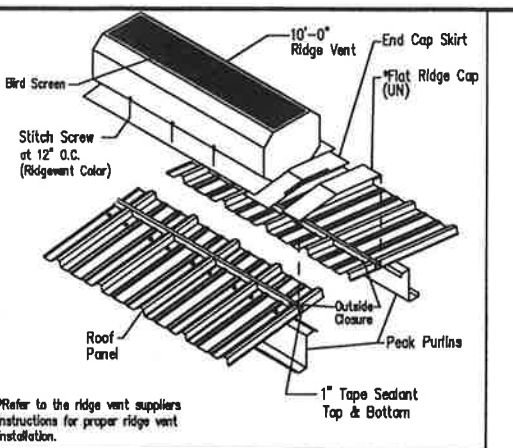
STANDARD PEAK BOX DETAIL

TRIM_100



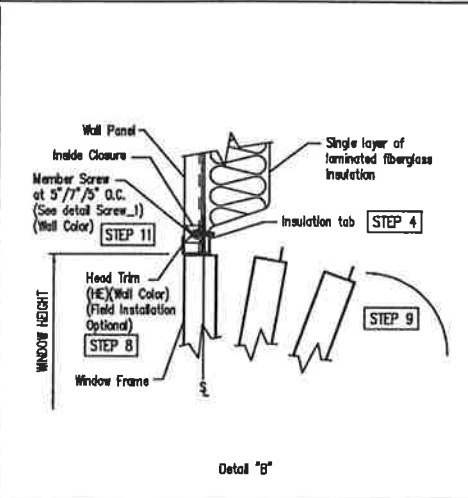
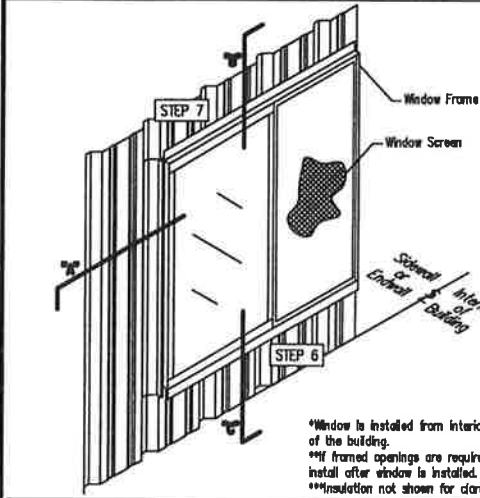
RIDGE DETAIL WITH FLAT RIDGE

TRIM_101



RIDGE DETAIL WITH FLAT RIDGE

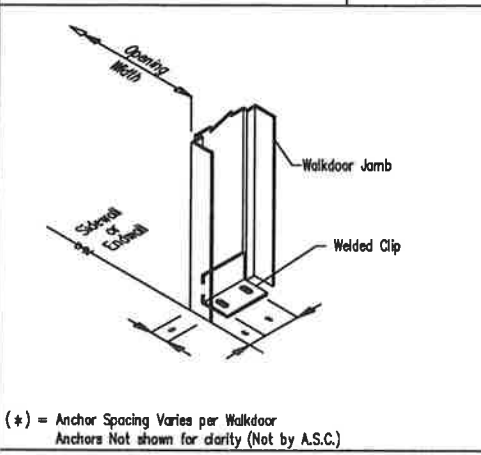
ACC_3



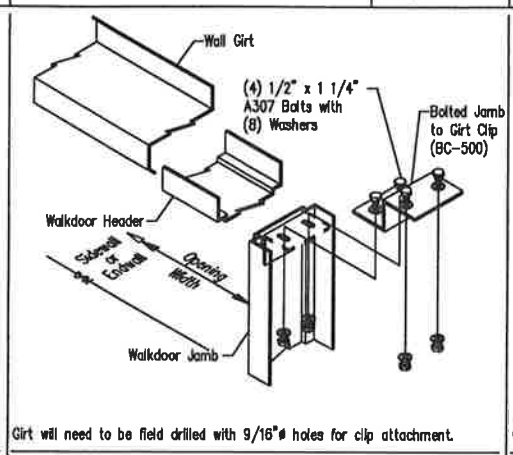
Detail "B"

- STEP 1** Install insulation (if required) and wall sheeting along entire wall.
- STEP 2** Slide panel fins, that are packaged with the window, down along the groove joints on each side of the window. Be sure that the bottom of each fin is flush with the bottom of the window frame.
- STEP 3** Determine window location and mark area to be cut. NOTE: Window width must fall on the high ribs of the wall panel(s).
- STEP 4** If insulation is required, cut and remove the fiberglass insulation from the laminated facing, approximately 4" from inside of the window opening. Then fold the laminated facing back over the top of the insulation. Creating a tab. Repeat for each side of the window.
- STEP 5** Starting at the top and working down towards the window height dimension plus an additional 1/8". Field cut the wall panel 1" from center of the high rib, towards the window frame, down each side of the window.
- STEP 6** Field cut the wall panel along the bottom of the window connecting the two previous lines.
- STEP 7** Field cut the wall panel along the top of the window.
- STEP 8** With the wall panel now removed, install the head trim and inside closure on the bottom of the wall panel along the top of the window opening by using some means of temporary support.
- STEP 9** Install the inside closure on the wall panel below the window. Place the bottom of the window frame on top of the wall panel. Sandwich the laminated facing tabs between the bottom of the window frame and wall panel, working your way up each side with the insulation tabs towards the top of the window. While rotating the top of window frame into place.
- STEP 10** Fasten the bottom of the window frame to the wall panel from the interior of the building.
- STEP 11** Fasten the top of the window frame to the wall panel from the exterior of the building. Then remove temporary support.
- STEP 12** Fasten the sides of the window frame to the wall panel from the exterior of the building.
- STEP 13** Caulk all sides of window framing against panel for weather tightness.

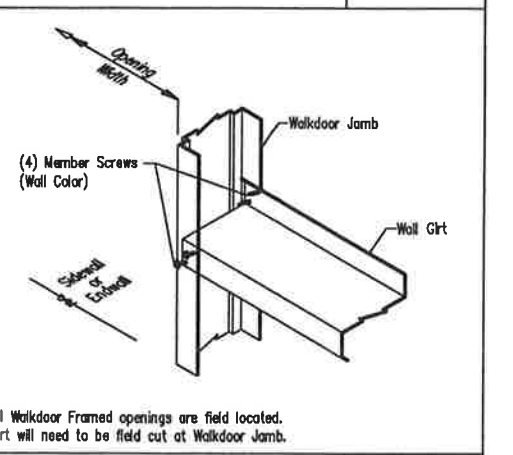
ACC_11



BASE DETAIL FOR WALKDOOR JAMB

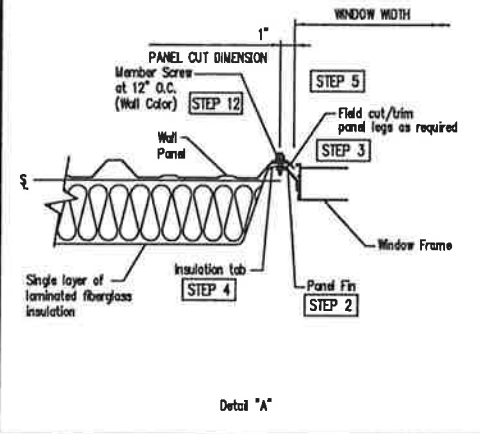


WALKDOOR JAMB TO WALL GIRTS

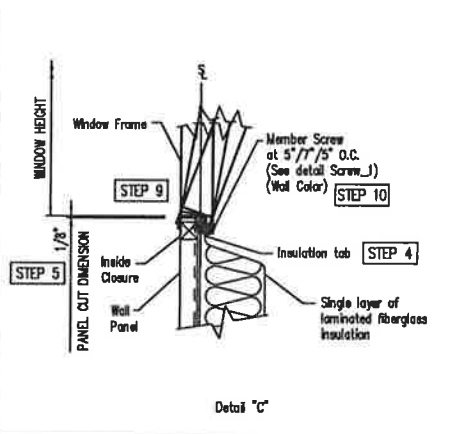


WALL GIRTS TO WALKDOOR JAMB

ACC_9

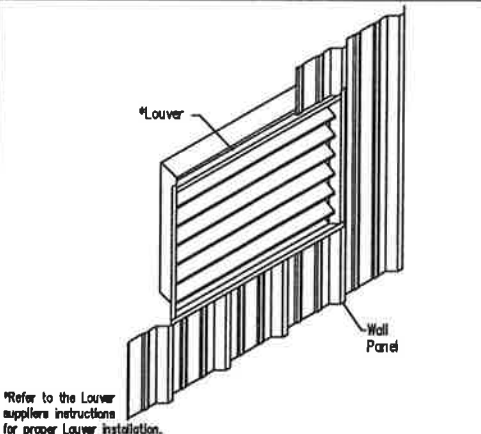


Detail "A"



Detail "C"

NON-THERMAL INSULATED WINDOWS



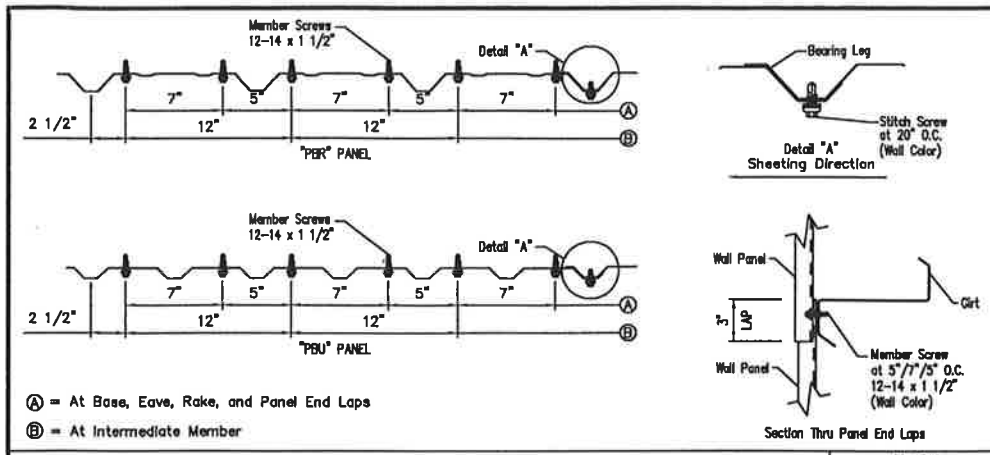
WALL LOUVER DETAIL

ACC_8

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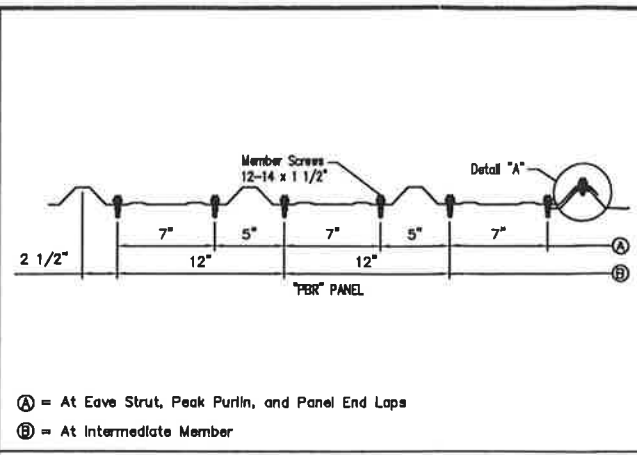
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JOB NO.:	55101
ENR. BY:	SS
DATE:	04/25/17
DWG. NO.:	15 OF 16
ISSUE:	E



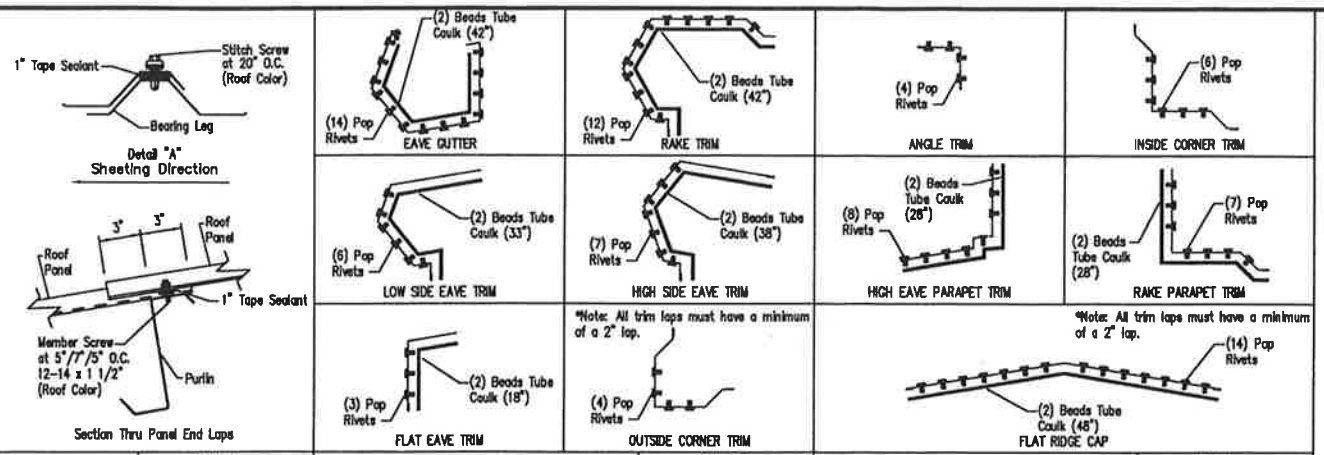
FASTENER LOCATION FOR WALL PANELS

SCREW_1



FASTENER LOCATION FOR ROOF PANELS

SCREW_2

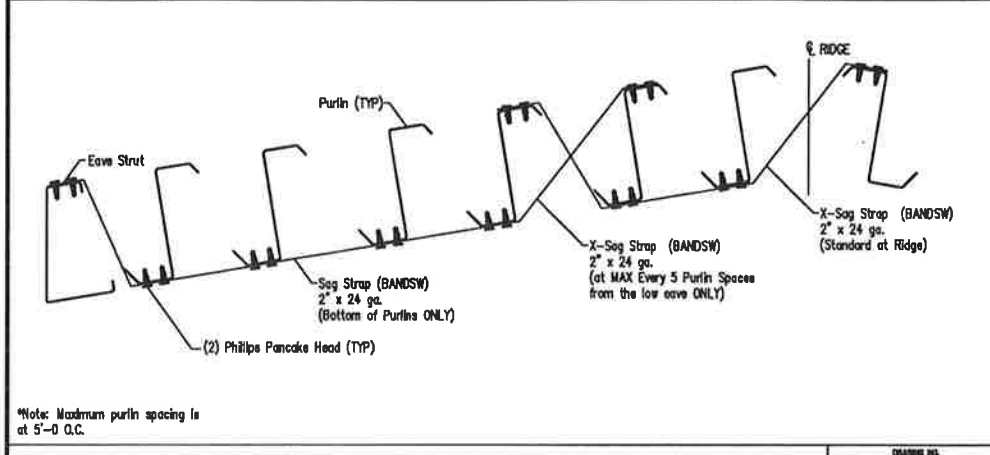


TRIM LAPS

SCREW_10

TRIM LAPS

SCREW_11



TYPICAL SAG STRAP AT GABLED ROOF

SCREW_15

ACCESSORIES SCHEDULE

#	ID	QUAN	SIZE	PART DESCRIPTION	PAGE #
1	WN-1	2	3'-0" X 3'-0"	3030 HORIZ. SLIDER PBR PANEL	**
2	WN-2	1	6'-0" X 3'-0"	6030 HORIZ. SLIDER PBR PANEL	**
3	DR-1	2	3'-0" X 7'-0"	3070 WALKDOOR TYPE M WHITE	**
4	DR-1	2	3'-0" X 7'-0"	3070 8" DOOR FRAME KIT	**
5	OH-1	1	12'-0" X 12'-0"	Insult OHD	**
6	-	2	9" X 10'-0"	CONTINUOUS RIDGE VENT FLAT SKIRT GALV. W/ DAMPER	**
7	LV-1	1	3'-0" X 3'-0"	FIXED LOUVER 3FT. X 3FT. WHITE	**
8	WN-3	3	7'-0" X 1'-0"	7010 HORIZ. SLIDER PBR PANEL	**

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JOB NO. 55101	DATE: 04/25/17
ISSUE: SS	ISSUE: E
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